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Geology and $F - 7$ Ground-Water Resources of the **Santa Maria Valley** Area, California

 B_Y G. F. WORTS, IR. With a section on Surface-water Resources by H. G. THOMASSON. IR.

GEOLOGICAL SURVEY WATER-SUPPLY PAPER 1000

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(GEOLOGY AND GROUND-WATER RESOURCES OF THE SANTA MARIA VALLEY AREA, SANTA BARBARACOUNTY, CALIF.

By G. F.WORTS, JR.

ABSTRACT

This report is the third in a series of interpretive reports on the several ground-'water basins of Santa Barbara Oounty, Oalif., prepared by the United States Geological Survey, in cooperation with the county. It presents the pertinent :,results of an investigation of the geology and water resources of the Santa Marla Valley area. It deals with the valleys of the Santa Maria and lower Sisquoc Rivers situated principally in the northwestern part of the county, which together 'form one large agricultural district dependent for its water supply on irrigation "from wells. The report presents data on runoff from the region tributary to the ,area, shows the extent to which ground water is replenished from the rivers, estimates total recharge to and total discharge from the one principal groundwater body, estimates the yield of that body, and discusses the possibility of sea-water encroachment.

. The, Santa Maria River, which is formed by the confluerice of the Ouyama Rivet and the Sisquoc River, flows generally westward to the Pacific Ocean. The Cuyama River, which enters the area from the north, and the Santa Maria River 'together form the boundary between San Luis Obispo and Santa Barbara Counties. Channels of the Santa Maria and lower Sisquoc Rivers overlie a large irregular i structural downfold or syncline, which is bounded on the north by the northwesttrending San Rafael Mountains and on the south by the west-trending Solomon and Casmalia Hills. These ranges are composed of consolidated essentially non-water-bearing rocks, which include the Franciscan, Knoxville (?), Monterey, Sisquoo, and Foxen formations, ranging in age from Jurassic to upper Pliocene. 'These impermeable rocks underlie the ground-water basin and bound it on the together form the boundary between San Luis Obispo and Santa Barbara Counties.

Channels of the Santa Maria and lower Sisquoc Rivers overlie a large irregular

Structural downfold or syncline, which is bounded on the north

The valley area between the bordering ranges consists mostly of broad terraced wplands and alluvial plains adjacent to the Santa Maria and Sisquoc Rivers. Beneath the uplands and plains and along the flanks of the ranges are the unconsolidated or water-bearing materials which have been deposited on the consolidated rocks, and which in part have been downfolded in the syncline. The "unconsolidated deposits are of upper Tertiary and Quaternary age, and attain a maximum thickness of about 3,000 feet. From oldest to youngest they include seven units: The Careaga sand, Paso Robles formation, Orcutt formation, terrace deposits, alluvium, river-channel deposits, and dune sand. Of these the alluvium of Recent age is the most permeable and yields water to more than 500 wells at rates of more than 1,000 gallons per minute per well.

Contained within these deposits and extending over an area of about 110,000 acres is a single large ground-water body. Near the coast over an area of 30,000 acres it is confined beneath silt and clay composing the upper part of the alluvium; over the remaining 80,000 acres it is unconfined. All ground-water recharge Copy of document found at was seen the unconfined portion or intake area. The chemical quality of the value of the unconfined portion or intake area. The chemical quality of the

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ground water is such that it can be used for most purposes, and further, analyses of well waters show that near the coast there has been no sea-water encroachment to date. During historic time there has always been a fresh-water head accompanied by ground-water outflow at the coast.

Recharge to ground'water is derived from seepage losses from streams and infiltration of rain. Seepage losses from streams for the 16-year period 1930-45 was estimated from the records of surface-water runoff which occurs over the 1,800 square miles of the drainage basin. Eight gaging stations on the Cuyama and Sisquoc Rivers and their major tributaries record the surface-water inflow to the valley area, and one station on the Santa Maria River at Guadalupe measures the surface-water outflow from the area. Infiltration of rain has been estimated on the basis of type of land cover and character of underlying deposits. estimated on the basis of type of land cover and character of underlying deposits,
using estimates of deep penetration of rain derived from work done mostly in
nearby Ventura County. Recharge from both sources has averaged nearby Ventura County. Recharge from both sources has averaged about 70,000

Discharge of ground water is by pumping and by natural means. Pumping for' irrigation, which began in 1898, constituted nearly 8'0 percent of the total discharge in 1944 when about 35,000 acres of land were irrigated from 317 wells. Estimates of pump age for the period 1929-44 were obtained largely from the kilowatt-hours used and electrical energy needed to pump 1 acre-foot of water. The total pumpage has increased from about $55,000$ acre-feet in 1929 to nearly $80,000$ acre-feet in 1944. However, the net pumpage is estimated to be about 20 percent less. Natural discharge during the period 1929-44 has been in the form of groundwater outflow to the sea from beneath the confining beds of the upper part of the alluvium. Outflow has ranged from about 9,500 acre-feet in 1936, when water levels and storage were the lowest of record, to nearly 13,000 acre-feet in 1944';

Increases and decreases in ground-water storage have been roughly proportional to periods of above-average and below-average rainfall, respectively, but have been modilied considerably by pumping during the past 20 years. The. period 1929-36 was one of below-average rainfall in which recharge averaged only about 34,000 acre-feet a year. By 1936 storage was depleted and water levels were lowered to the point where pumping lifts locally became economically infeasible. The net decrease in storage in this period is estimated to have been 160,000 to 200,000 acre-feet. The following period, 1936~45, was one of average rainfall in which recharge averaged nearly 100,000 acre-feet a year. $\overline{1n}$ the heavily pumped area the average net rise in water levels amounted to 30 feet, and the over-all net increase in ground-water storage was about 260,000 acre-feet. \blacksquare

The perennial yield of the ground-water basin for the period $1929-45$ was estimated by two independent methods, as follows: It is equal to the total recharge less the total natural discharge divided by the 16 years of inventory, and it is equal to the total net pumpage plus the net increase in storage divided by the 16: 'year's inventory. The yield for the period is considered to be the average of .the, two, but because of somewhat greater than average rainfall it is probably slightly greater than the long-term average. Based on a comparison with rainfall for the period 1886-1945, the perennial yield is estimated to be about 53,000 acre-feet' year. Current net pumpage is about $65,000$ acre-feet a year, and therefore, the perennial yield is being exceeded by about 12,000 acre-feet a year.

A program outlined by the Bureau of Reclamation to utilize more efficiently the surface-water resources of the Santa Maria River drainage system involves the construction of dams to detain the surface-water inflow, to transfer water from the reservoirs to the ground-water basin by natural spreading in the permeable channels, and so to salvage a considerable part of the estimated $33,000$ acrefeet a year now wasting to the sea as surface-water outflow. Under such a program the perennial yield could be increased in nearly direct proportion to the quantity of outflow salvaged.

INTRODUCTION

LOCATION AND GENERAL FEATURES OF THE AREA

The Santa Maria Valley area is one of the larger coastal valleys of California, and is situated about 130 miles northwest of Los Angeles and 60 miles northwest of Santa Barbara. It occupies the northwestern part of Santa Barbara County and the extreme southwestern part of San Luis Obispo County (fig. 1). It lies approximately

FIQUE 1.—Index map of Santa Barbara County, California, showing location of the Santa Maria River' . drainage system'

between 34°50' and 35°5' north latitude, and between 120°10' and 120°40' west longitude (pl. 1). It covers an area of about .260 square miles and has an east-west length of 28 miles and a maximum north-south width of 15 miles.

This area comprises the alluvial plains and adjoining terraces, foothills, and mountain slopes of the Santa Maria Valley and of the lower walley of the Sisquoc River. The Santa Maria River is formed by the way, of document found at was separated the Sisquoc and Cuyama Rivers at Fugler Point (pl. 1) and flows westward across a broad alluvial plain, called the Santa

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Maria plain, to the Pacific Ocean. A small alluvial plain adjoins the Santa Maria plain at Fugler Point and extends up the Sisquoc River to La Brea Creek. The' Cuyama River, though longer than the' Sisquoc and draining a much larger area, has developed no appreciable alluvial plain within the area here considered. Bordering the Santa. Maria plain on the north and south are relatively elevated terrace areas referred to as the Nipomo upland and the Orcutt upland,. respectively. The Nipomo upland borders and rises gently northward. to the westward extension of the San Rafael Mountains; the Orcutt· upland borders and rises gently southward to the Solomon and Casmalia Hills. Most of the plains, and little of the upland areas, areextensively cultivated and represent the largest single agricultural. district in Santa Barbara County.

Between the mountains, which are composed mainly of consolidated rocks, the uplands and alluvial plains as a whole are underlain by alarge mass of unconsolidated deposits which contain a single, large, ground-water body. This body, herein designated the' main water, body, supplies water to more than 700 irrigation, public supply, and. industrial wells whose aggregate net draft in 1944 was about 65,000' acre-feet.

HISTORY AND CULTURAL DEVELOPMENT

The principal community in the area is the city of Santa Maria. (originally named Central City), founded in 1876 , and situated on the Santa Maria plain 12 miles from the coast. It lies astride U. S. Highway 101 at its junctions with State Highway 166. Nine miles; to the west is the town of Guadalupe, founded in 1872, and situated. on State Highway 1 and on the coastal line of the Southern Pacific railroad, which was completed in 1901. The Santa Maria Valley Railroad connects Santa Maria and the sugar beet refinery at Betteravia with the Southern Pacific railroad at Guadalupe. About 5 milessouth of Santa Maria is the small oil town of Orcutt on State Highway' 1; and 6 miles north of Santa Mariais the small agricultural communityof Nipomo on U.S. Highway 101. On the plain of the Sisquoc River 9^o miles southeast of Santa Maria are the small towns of Garey and. Sisquoc, both on State Highway 140.

The first settlers were the Spanish in about 1840. The large land grants established thereafter are shown on plate 1. Among thelarger of these are the Ranchos Guadalupe, Nipomo, Punta de la-Laguna, Tepusquet, and Sisquoc; American pioneers arrived in about' 1865 and since then have purchased most of the land comprising the old Ranchos. The early Spanish settlers raised mostly cattle and. feed, but during the drought of 1863–64 they lost dapp of document found at when NoNewWipTax.com liquidation of the large grants began,

USE OF GROUND WATER

Essentially all the inigated acreage, the' major industries, and all public water-supply systems depend upon water from wells which tap the large ground-water reservoir, or main water body. By far the greatest demand upon this reservoir is made by truck farming. Over 300 irrigation wells supply water to about 35,000 acres of land. Upon this land one or two crops of lettuce, cauliflower, carrots, or other . vegetables are raised each year,' In addition, alfalfa, flowers for seed, and sugar beets are raised. The sugar beets are processed at the refinery at Betteravia, which is supplied with water from a battery . of 10 wells along the north edge of Guadalupe Lake.

The city of Santa Maria derives its water supply from three wells about 4 miles south of the city. Also, the towns of Guadalupe, Orcutt, Betteravia, and Sisquoc derive their water supply from wells. The towns of Garey and Nipomo have no public water-supply systems and obtain their water from domestic wells, as do the numerous farms throughout the area. farms throughout the area.
There are several major oil fields in the area as follows: the Santa

Maria Valley oil field, immediately south of and extending both east and west from the city of Santa Maria; the Orcutt oil field, on the crest of the Solomon Hills due south of the town of Orcutt; and the Cat Canyon oil fields about 2 miles south of the town of Sisquoc. Most of the oil produced by the major companies is transported out of the area in crude form by truck, rail, or pipeline. However, near the city of Santa Maria there are several small refineries that process a considerable quantity of oil. Water used in the refining process and in oil-field operations is furnished entirely from wells.

Other principal industries in the area that depend upon groundwater supply are the vegetable-packing plants and the ice-manufac--turing plants at Guadalupe and at Santa Mana,

PURPOSE AND SCOPE OF THE INVESTIGATION AND REPORT

The investigations of which this report is the third were begun by the Geological Survey, United States Department of the Interior, in January 1941 in cooperation with Santa Barbara County. The first two parts deal with the Santa Ynez River valley and the south-coast basins, respectively, and results are embodied in two reports by Upson and Thomasson (Upson and Thomasson, 1951; and Upson, 1951/, This report gives the results of the investigation in the Santa Maria Valley area. It has been carried on by the Geological Survey, United States Department of the Interior, under the. direction of O. E. Meinzer, geologist in charge of the Ground Water Branch; and under

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the general supervision of A. M. Piper, district geologist in charge of ground-water investigations on the Pacific coast.

The investigation was begun in 1941 with the general objectives of estimating the yield of the ground-water basin supporting irrigation, and of evaluating the possibility or presence of sea-water contamination of the ground-water bodies. The investigation was also related to. broad plans¹ for the county-wide utilization of water resources, under which it is proposed to construct reservoirs on the Cuyama and Sisquoc Rivers for purposes of controlling floods, and storing or detaining flood waters that can be released for replenishment of ground-water reservoirs downstream. Thus, the investigation was also directed toward the solution of problems pertaining to the amount and distribution of runoff in the two rivers and to replenishment of ground-water bodies from them.

Accordingly, the ground-water and surface-water phases of the work had somewhat different scopes. The detailed study of the geology, ground-water conditions and resources, and river-seepage losses was restricted to the Santa Maria Valley area, as here defined and shown on plate 1; whereas the study of runoff concerned the entire drainage basins of the Cuvama and Sisquoc Rivers, a total area of about 1,600 square miles, shown on plate 4.

Specifically, this report describes the geology of the Santa Maria. Valley area as it pertains to the occurrence of ground water; the report summarizes the runoff from the Cuyama and Sisquoc River drainage basins, and estimates the seepage losses to ground water in the lower Sisquoc, Cuyama, and Santa Maria River channels; it describes the occurrence, source, movement, and natural discharge of ground water: it estimates the amount of discharge, both natural and artificial; it discusses the quality of the ground water and the possibility of seawater contamination; and it estimates the perennial yield of the ground-water basin.

The collection of basic ground-water data was begun by the Geological Survey in the spring of 1941 and has been carried on to date. Records of stream flow were made in 1903-05, and from 1929 to date, The geologic field work was begun in January 1944 and carried on intermittently until September 1945.

¹ Water resources and utilization, Santa Maria, Santa Ynez, and related basins, U. S. Dept. Interior, Bur, Reclamation, Harry W. Bashore, Commissioner, C. E. Carey, Director of region 2: Mimeographed project planning rept. No. 2-3.1-3, pp. 27-29, June 1945.

CLIMATE

The climate of the Santa Maria Valley area is characterized by a wet and a dry season. The average annual rainfall over the area varies considerably, but in general the lowland areas receive less rain than the surrounding mountains. About 95 percent of the rainfall occurs during the seven months from October through April, during which time the heaviest rainfall originates from storms moving in from the Pacific Ocean.

The temperature varies considerably between winter and summer, but the mean annual temperature is about 60° F. During the winter temperatures below freezing are infrequent and usually occur during the night. On the other hand, the summers are mild with temperatures usually in the 70's. Only on the rare occasions when hot winds sweep seaward from the valleys of central California does the temperature approach 100° F.

The prevailing winds are from the northwest, and during the summer months these winds bring heavy fogs which extend like long white fingers into the coastal valleys. The fog usually appears in the evening and lasts until about noon the following day, at which time it is "burned off" by the sun. Because the fog acts as an insulator against heat from the sun, it is beneficial to some types of crops.

Records of rainfall and of other detailed climatological data for Santa Barbara County have been presented in another report (Upson, Water-Supply Paper 1108, in preparation). However, there are presented in table 1 records of rainfall at six stations whose locations are shown on plate 4. The records show the seasonal distribution, the monthly quantities, and the variations in quantity of rainfall with altitude.

TABLE 1.-Monthly and yearly precipitation, in inches, at 6 stations in the Santa Maria Valley area, California

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TABLE 1. Monthly and yearly precipitation, in inches, at 6 stations in the Santa María Valley area, California-Continued

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PREVIOUS INVESTIGATIONS

The geology of the Santa Maria Valley area was first observed casually in the early 1850's during the explorations and surveys for the Pacific Railroad. Other examinations of a reconnaissance type were made in the latter part of the 19th century. H. W. Fairbanks (1904) presented a comprehensive report on the geology of the San Luis quadrangle, and in his work touched upon the general geologic features of the Santa Maria Valley area. These general geologic features are shown on the California geologic map (Jenkins, 1938), and have been described by Reed (1933).

Arnold and Anderson (1907) were among the first men to study the geology to determine the oil resources of the area. In that, and in subsequent reports of a similar nature, particular emphasis was placed on the potential and proved oil-producing structures and the reservoir rocks near and beneath the Santa Maria Valley. These studies have been made in recent years by Frame (1938), Canfield (1939), and Woodring (Woodring, Bramlette, and Lohman, 1943). Also, summaries of the local oil fields have been presented in a report by the California Division of Mines (1943, pp. 235-238 and 430-442).

The only hydrologic report on the area is one the late J. B. Lippincott² submitted to the Santa Barbara County in 1931. In addition to the hydrologic study, the report deals with the feasibility of building dams across the Cuyama and Sisquoc Rivers for the dual purpose of flood control and water conservation.

ACKNOWLEDGMENTS

The writer is grateful for the cooperation and assistance given by many persons and several agencies in the Santa Maria Valley area. The Santa Maria Valley Water Conservation District made available records of water-level measurements, drillers' logs, and chemical analyses. In addition, this organization supplied the map of the area. from which the base for plate 1 was compiled. Information on the subsurface geology of the area was obtained from numerous oil-well. logs and other data supplied by Mr. L. N. Waterfall, Mr. A. W. Hughes, and Mr. Charles Manlove of the Union Oil Co. of California, and by Mr. S. G. Dolman of the California State Division of Oil and Gas. Mr. G. V. Footman, District Manager, and other officials of the San Joaquin Power Division of the Pacific Gas & Electric Co. gave freely of their time in supplying records of power consumption, pumptests, and water-level measurements. Dr. M. G. Edwards of the Shell Oil Co. of California furnished chemical analyses of well waters.

I Lippincett, J. B., Report on water conservation and flood control of the Santa Maria River in Santa While EaX LGPO bispo Counties, Calif., March 1931 (unpublished report available to the public at: the offices of the County Planning Commission, Santa Barbara, Calif.).

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Mr. York Peterson, engineer of the city of Santa Maria, and Mr. A. A. Howard, water plant superintendent, furnished records of water-level measurements and of pumpage for the city pumping plant,

Valuable information and time were donated by Mr. C. J. Longwell, a water-well driller, and Mr. W. C. Matthews, Byron-Jackson pump dealer, both of whom supplied many water-well logs. The cooperation and support given by ranchers and residents were greatly appreciated.

The portion of the geology shown on plate 1 and lying south of the Santa Maria and Sisquoc Rivers is in large part after Woodring. (Woodring, Bramlette, and Lohman). Much valuable information is. presented in Lippincott's ³ report, and it has been of great value in analyzing the hydrologic conditions of the area.

The writers acknowledge the advice and criticism of their colleagues of the Geological Survey in the preparation of this report. The report as a whole was improved by J. F. Poland and J. E. Upson, the section on quality of water was amended by A. A. Garrett, and the section on pumpage was improved by Penn P. Livingston, all members of the Ground Water Branch. The section on surface-water resources was improved by H. M. Stafford of the Surface Water Branch.

.. GEOLOGY

LAND FORMS

The Santa Maria Valley area is primarily the topographic expression' of underlying geologic structures modified by the action of streams and rivers. The valley area overlies a broad downfold, or syncline; the bordering hills and mountains are the surface expression of anticlines or regional uplifts. The northwest-trending extension of the San Rafael Mountains is chiefly an uplift and forms the north border of the valley area; the Solomon and Casmalia Hills are on the axes of upfolds and form the south border. Between these ranges is the broad lowland occupied by the valleys of the Sisquoc and Santa Maria Rivers. This is the agricultural district and has a somewhat varied topography, many of whose features reveal elements of the geologic history or affect the occurrence and utilization of ground water.

LOWLANDS

Alluvial plains.-There are two major alluvial plains in the area, one in the Santa Maria Valley and the other in the Sisquoc valley and they are herein designated the Santa Maria plain and the Sisquoc plain, respectively (pl. 1). Along the Cuyama River, owing to its constriction in a relatively narrow consolidated rock gorge within the area, are only small remnants of an alluvial plain, which are relatively at way. NoNewWipTax.com

² Lippincott, J. B., op. cit., 1931.

unimportant by comparison with those of the valleys of the Santa GEOLOGY 19

Maria and Sisquoc Rivers.

The Santa Maria plain extends from Fugler Point on the east to

the sand dunes and the Pacific Ocean on the west-a distance of about .20 miles. It includes the wedge-shaped part of the alluvial plain lying northwest of the river in San Luis Obispo County, which is known locally as the Oso Flaco district. The plain attains a maxi-. mum width of more than 5 miles in the vicinity of Guadalupe and has an area of about 36,000 acres. It is a gently inclined, nearly level surface, which reaches a maximum elevation of 350 feet at Fugler Point and has an average westward gradient of about 17 feet. per mile. It is the principal irrigated agricultural district in the area, and is supplied with water by nearly 300 irrigation wells.

Along the south side of the plain, and extending from U. S. Highway 101 to the mouth of the Santa Maria River, is an old channel of the river known as Green Canyon (pl. 1). Because it has been an inactive channel during historic time and, further, because it has been under cultivation for many years, it is herein considered as a part of the alluvial plain.

The Sisquoc plain begins at La Brea Creek, at an elevation of about 540 feet, and extends downstream along the south side of the river to Fugler Point-a distance of about 8 miles. It has a maximum width of about 3,500 feet in the vicinity of the town of Sisquoc, is a relatively flat surface which has a gradient of about 24 feet per mile, and slopes .slightly both downstream and toward the river. The surface area 18 a little more than 2,000 acres, most of which is irrigated by 17 wells. River channels.—The Sisquoc, Cuyama, and Santa Maria Rivers all maintain relatively wide and distinct channels within the limits of the area (pl. 1). The Sisquoc and Cuyama Rivers join at Fugler Point to form the Santa Maria River. Their channels are essentially dry washes supporting little or no vegetation; they have appreciable flow only during the wet winter months, and then mainly during floods.

The Santa Maria River channel is approximately 22 miles long and has an average seaward gradient of about 15¹/₂ feet per mile. The lower courses of the Cuyama and Sisquoc Rivers have gradients of. about 19 and 24 feet per mile, respectively. The channels range in width from a minimum of 750 feet at the mouth of the Cuyama River to a maximum of 7,000 feet northwest of Santa Maria. The combined surface area of the Sisquoc and Santa Maria River channels is lower courses of the Cuyama and Sisquoc Rivers have gradients of about 19 and 24 feet per mile, respectively. The channels range in width from a minimum of 750 feet at the mouth of the Cuyama River to a maximum of 7,000 f about $11,000$ acres—about one-quarter of that of the adjacent alluvial plains.

New Young Taxface of the channels of the Sisquoc River and of most of the Santa:Maria River is only 3 to 5 feet below the surface of the alluvial

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plains, and in times of extreme flood the rivers extend laterally onto the plains, causing extensive property damage and depositing silt, and sand over the arable land. For example, during the flood of February 9, 1915, the water extended into the city of Santa Maria. Near Guadalupe, however, the Santa Maria River has entrenched itself from 10 to 20 feet below the surface of the plain, and is usually 'confined by its banks.

At the western end of the Santa Maria Valley, the river formerly had two outlets to the ocean through the dune sand deposits-one through Oso Flaco Lake along the north edge of the yalley, which in recent years has been blocked; the other, farther south and west of Guadalupe. The abandoned channel leaves the present, channel about 3 miles upstream froni Guadalupe and follows the course, of 'Oso Flaco Oreek, which drains'that portion' of the Santa Maria plain " lYing in San Luis Obispo Oounty, and which empties into Oso Flaco Lake (pl. 1) . Because the creek has insufficient discharge to maintain an opening to the sea, drainage from the lake into the Pacific Ocean takes place by seepage through the sand deposits that separate :them.

The present outlet of the Santa Maria River is blocked by beach 'sand during the summer months. The shallow lakes which form be~ hind the beach bar are supplied with water by discharge from a minor water body $(p, 74)$. Only during the winter months when the river is at a relatively high stage is there a direct connection between the river and the ocean.

Terrace surfaces.--The terrace surfaces occur between the alluvial plains and the bordering hills and mountains, and are often referred to locally as "mesas" or "uplands." These are stream-formed features. quite distinct from but correlative with the numerous smaller remnants of marine terraces along the coast.

Inland from the ocean the terraces occur at two general levels, 40 feet and 100 feet above the adjoining alluvial plains, and they are hereafter referred to as the 40 -foot terrace and the 100-foot terrace, respectively. Of the two, the 40-foot terrace is the younger, the better, developed, and the more widespread. In the canyon of the Sisquoc River it is locally over 50 feet above the river channel. The 100-foot terrace is less extensive and is poorly developed; it is best obs southeast of Nipomo. However, west of Nipomo Creek it is covered: by dune sand. '

South of the Santa Maria plain the surfaces of the two terraces plus the large area of dune sand together form the Orcutt upland. North of the Santa Maria plain the large area of dune sand and the 100-foot terrace together form the Nipomo upland α . Hocument found \mathbb{R}

. Fairbanks (1904, pp. 12 and 13) noted remnants of 10 marine terraces at heights of 10, 40, 60, 80, 100, 200, 350, 570, 700, and 750 feet :above sea level. Of these the 40-foot and 100-foot terraces are prob ably the marine equivalents of the alluvial terraces in the Santa Maria Valley area.

Sand dunes.-The sand dunes on the Nipomo and Orcutt uplands and at the west end of the Santa Maria plain form another prominent topographic feature of the area. The sand dunes form a very irregular but typical topography. The prevailing northwest wind is and has been the controlling agent in their formation and has elongated them in a northwest-southeast pattern, with numerous narrow closed drainage basins lying parallel to and contained between the ridges. The have gentle slopes on the northwest or windward side and steep slopes on the southeast side, where the drifting sand spills over onto the lee side of the dunes. When the wind is blowing hard the rate of sand spilling over the lee side of the active or modern dunes becomes Trapid and the dunes are said to be "moving" or "drifting." In this manner the dunes have "moved" inland from the beach and are continuing to do so.

BORDERING HILLS AND MOUNTAINS

San Rafael Mountains.-The most prominent mountain range in Barbara County is the San Rafael Mountains. Big Pine Mounwhich rises to an altitude of 6,828 feet, is the highest peak in the range and also the highest peak in the county. It is about 30 miles ϵ east-southeast of the area. From Big Pine Mountain the crest of the range decreases in altitude gradually northwestward. It forms the northern boundary of the Santa Maria Valley area, and there has an altitude of $1,700$ to $3,000$ feet.

The core of the range, which is composed of old and resistant rocks, is highly dissected and is characterized by deep ravines and knife-edge ridges jutting off at sharp angles to the axis of the range. This jagged topography is further emphasized by large fault escarpments. Adjacent to the area, however, the topography is less rugged for in general the rocks are younger and less resistant. Plate 1 shows four major streams heading in the range. From east to west they are La Brea and Pepusquet Creeks, which are tributaries of the lower Sisquoc River; and Suey and Nipomo Creeks, which are tributaries of the Santa Maria \mathbb{R} iver.

The courses of the Sisquoc and Cuyama Rivers are outstanding . physiographic anomalies. The Cuyama River, which flows in a westerly direction on the north side of the Sierra Madre and the San Rafael Copy of document found # \mathcal{L} (Mount balias. (pi). 4), turns southward immediately below its junction with Huasna Creek and crosses the axes of the ranges to join the Sis-

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quoe River at Fugler Point. The Sisquoc River flowing westward heads between the two ranges and crosses the San Rafael Mountains at an oblique angle east of the area shown on plate 1. Consequently, the drainage area of the rivers covers parts of both the north and south sides of the Sierra Madre and the San Rafael Mountains. No part of the Sisquoc drainage area is north of the Sierra Madre.

Casmalia and Solomon Hills.-The Casmalia and Solomon Hills. whose crests form the southern drainage divide of the area, are essentially one continuous range of hills extending westward from their junction with the San Rafael Mountains near Foxen Canyon to the Pacific Ocean (pl. 4). These hills are separated by a low saddle at an altitude of 520 feet, known as Graciosa Divide: .

The Solomon Hills reach a maximum altitude of about 1,620 feet south of the town of Sisquoc. The hills consist of a moderatelyresistant anticlinal core of Miocene and Pliocene shales whose topography is characterized by steep ravines and knife-edged ridges. The flanks are composed of relatively unconsolidated upper Pliocene and Pleistocene gravel, sand, and clay whose topography is characterized. by rolling hills and moderate to deep gullies. Heads of many canyons;. and larger gullies are amphitheatric in shape-a feature brought about by landslides...

The Casmalia Hills are similar to the Solomon Hills in most physiographic respects. Mount Lospe is the highest peak and rises to an an altitude of about $1,640$ feet-only 20 feet higher than the highest peak in the Solomon Hills. Northwest of Shuman Canyon, the Casmalia Hills veer to the northwest and older basement rocks crop. out. . Arnold and Anderson (1907, p. 19) describe this part of the hills. as follows; ,

The Casmalia Hills, particularly that portion north of Schumann [Shuman]' Canyon, have a distinct individuality among the topographic features of the basin region, and may be regarded as a separate although small range allied in age and character with the bounding ranges. It is conformable in trend with the Sanl Rafael Mountains and forms a prominent headland jutting out to sea.

"

This prominent headland, formed by the resistant core of the range. is Point Sal; its impressive cliffs, rise as high as 1,000 feet above the' sea. Both north and south of the point, wave action has eaten more. rapidly into the less resistant rocks composing the flanks of the range, and cliffs are less pronounced and gradually merge with the valley plains. Although the headlands suggest a coast line of emergence theadjacent valley fills of Recent age indicate that the coast line in reality is one of submergence in Recent time.

From east to west the principal streams draining the north flanks of the Casmalia and Solomon Hills are in Foxen Canyon, Odec Canyon, dat www.NoNewWipTax.com

Stratigraphic units of the Santa Maria Valley area, California

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Solomon. Graciosa, and Shuman Canvons. Owing to the relatively light rainfall on these hills, the runoff of the streams is ephemeral and extremely low.

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GEOLOGIC FORMATIONS AND THEIR WATER-BEARING PROPERTIES

AGE AND DISTRIBUTION

For the purposes of this report, the geologic formations in the Santa Maria Valley area have been divided into two groups: Unconsolidated water-bearing deposits which are of uppermost Tertiary and Quaternary age: and consolidated and essentially non-water-bearing rocks. which underlie the unconsolidated deposits and which range in age from Jurassic to upper Tertiary. From oldest to youngest—that is. in succession upward—the unconsolidated deposits include the Careaga sand, the Paso Robles and Orcutt formations, the terrace deposits, alluvium, river-channel deposits, and dune sand. They consist largely of lenticular bodies of gravel, sand, silt, and clay. They occur in the central part, or heart of the area, in an asymmetric structural depression or synclinal trough extending from La Brea Creek to the ocean. Canfield (1939, p. 69) has designated the part of the syncline in the Santa Maria Valley as the Santa Maria Valley syncline; its axis is shown on plate 1. Along this axis, near Orcutt, the unconsolidated deposits attain a maximum thickness of about 3,000 feet and extend westward beneath and are in contact with the Pacific Ocean. From oldest to youngest, the consolidated rocks include the Franciscan and Knoxville (?) formations, the Monterey shale and interbedded volcanics, the Sisquoc formation, and the Foxen mudstone. These consolidated rocks form the north, east, and south sides and the bottom of the ground-water basin.

Oil well logs show that the Tertiary and Quaternary rocks attain a maximum thickness of more than 10,000 feet along the axis of the Santa Maria Valley syncline near Orcutt, and that they thin with moderate rapidity up the flanks of the syncline to the north and south before cropping out in the surrounding hills and mountains.

Plate 1 shows the areal distribution of the various formations; plate 2 shows their stratigraphic and structural relations; and the following table of stratigraphic units summarizes their sequence, general characteristics, and water-bearing properties.

Because the consolidated rocks are essentially not water bearing and are important only in that they define the basal and lateral limits of the main water body and its containing deposits, they are distinguished on the geologic map and cross sections (pls. 1 and 2) only as consolidated Tertiary rocks undifferentiated, and as the Franciscan and Knoxville (?) formations. On the other hand, the unconsolidated

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'deposits, or water-bearing formations, have' been mapped carefully and the areal extent of each is shown in detail on all three plates. The more detailed stratigraphic and structural relations and the lithologic character of the water-bearing formations are shown on plate 3 which has been' compiled from logs of water wells, The five geologic sections are along lines shown on plate 1.

CONSOLIDATED ROCKS FRANCISCAN AND KNOXVILLE (?) FORMATIONS (JURASSIC?)

The oldest recognized rocks in the area are the metamorphosed igneous and sedimentary rocks of the Franciscan formation, which is of Jurassic (?) age. Closely associated with these are sedimentary and metamorphic rocks, which possibly are partly of the Knoxville formation of Jurassic (?) age. Where examined in the north-central part of the area, both formations have been moderately to intensely folded and faulted, and no effort was made-to distinguish them in the field or on the geologic map (pl. 1). Woodring (Woodring; Bramlette, and Lohman, 1943, p. 1343), recognized both formations in a small area. in the western part of the Casmalia Hills, and found the Franciscan. formation to consist principally of altered basait and gabbro minor areas of peridotite and serpentine, and the Knoxville formation to consist of intercalated beds of sandstone, conglomerate, and darkcolored shale, which is locally altered to lustrous phyllite. Canfield, $(1939, pp. 67, 68)$ examined the cores from oil wells drilled in the Santa Maria oil field, and found the Knoxville formation to be posed of fractured calcite-veined hard greenish-gray pyritiferous medium-grained sandstone, highly faulted and slickensided clayshale. A few foraminifera were found in these cores which suggest a possible Cretaceous age for a part of the rocks encountered,

Where the Franciscan and Knoxville $(?)$ formations crop out along: the north side of the Santa Maria River, they were found 'to principally of fine-grained green sandstone, thin-bedded dense greenish to red chert, and slickensided light-green to dark-green serpentine, with lesser amounts of hard gray glaucophane schist, quartzite, and green to black shale.

MONTEREY SHALE AND INTERBEDDED VOLCANIC ROCKS (MIOCENE)

The Monterey shale is separated from the underlying Franciscan and Knoxville(?) formations by a major unconformity; which at places may be a fault. No other formation is known to occur between the Monterey shale and the Franciscan and Knoxville (?) formations in the area covered by this report. However, in the western Casmalia. Hills, $Wooding. (Woodring.;Branchette, and Lohman, 1943.1343-1345)$ mas, it courings (11 couring), *phalmotoc*, and gone copy of document found at was MoNewWipTax.com
mapped two intervening formations, the Lospe formation of lower

 M iocene $(?)$ age and the Point Sal formation of early middle Miocene age: Also, along the north edge of the valley there are possibly older rocks of Tertiary age.

The Monterey shale, which is of middle and upper Miocene age, is of marine origin, and is the principal source rock of petroleum. It attains a maximum thickness of about 7,000 feet in the structural trough beheath the town of Orcutt, but is considerably thinner elsewhere. It forms the core of the Casmalia and Solomon Hills, extends beneath the Santa Maria and 10wer'Sisquoc valleys at considerable depth, and rises to the north to form the main part of the-San Rafael Mountains shown within plate 1. It has been described by Woodring (Woodring, Bramlette, and Lohman, 1943, p. 1345) as follows:

Three mapped members are reoognized in the Monterey of the Santa Maria distriot. The lower member is characterized by phosphatic shale, silty shale, and ,somewhat 'pqr,celarieous, shale; the middle member by chert, cherty shale, and porcelaneous shale; and the upper member by porcelaneous shale, or by porcelane ous shale and soft diatomaceous strata. The lower member is 200 to 900 feet thick in the western Casmalia Hills; the middle member has an average thickness of 200 feet; and the thickness of the upper member varies from 600 to 700 feet in the western Casmalia Hills and is about 1,000 feet in the eastern Purisima Hills. Limestone, doubtless more or less dolomitic and presumably not of primary origin, is found throughout the formation, being most abundant in the lower member. The chert of the middle member is characteristically contorted and ,forms generally c0nspicuous outcrops. Wherever the upper member, includes)Joth hard poroelaneous shale and soft distomaceous strata, the soft diatomaceous strata overlie the hard porcelaneous shale.

In exposures the Monterey shale characteristically occurs in thin beds, 1 inch to 3 inches thick, which are usually white to light yellow in color, and highly jointed and fractured.

. The volcanic rocks associated with the Monterey shale differ widely in character from place to place, but in general fall into two classes-pyroclastic and intrusive rocks. At the mouth of the Cuyama River, Arnold and Anderson (1907, p. 34, 35 and pl. 3A) recognized the pyroclastic rocks, which were probably laid down under marine conditions. These deposits show distinct bedding, and are composed of resistant red and white agglomerate and yellow tuff beds with a few interbedded strata of sandstone and cherty limestone. They also crop out, south and east of the town of Nipomo.

The other type of volcanic rock associated with the Monterey shale is intrusive andesite. Outcrops of this material can be seen in a road .'cut on State Highway 166, just east of Suey Road. It is dark green to black in color and has a pillowlike structure, indicating that it .probably²was erupted:sDaqueouslysinto the plastic Monterey shale.

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SISQUOC FORMATION (MIOCENE AND PLIOCENE)

The Sisquoc formation is exposed high along the north flank of the Solomon and Oasmalia Rills. Also, it underlies the valleys of the. Santa Maria and Sisquoc Rivers and crops out along the north flank.' of Sisquoc Valley, but it laps upon the Franciscan and: Knoxville (?). formations beneath the Santa Maria Valley.

The Sisquo.c formation, which is of upper Miocene and lower and middle Pliocene age, rests unconformably upon the Monterey shale. It is represented by a coarse-grained shallow-water facies in the Sisquoc River valley, and by a fine-grained deep-water facies in the western part of the area. The deep-water facies attains a maximum thickness of about 3,000 feet and is composed primarily of massive' diatomaceous mudstone with some porcelaneous shale and claystone beds. The shallow-water facies is considerably coarser and thinner; and is composed of relatively hard beds of siltstone and some con-, glomerate. In surface exposures the 'Sisquoc formation resembles the . Monterey shale to a marked degree, particularly where the deep-'water facies of the formation is represented. '

FOXEN MUDSTONE (PLIOOENE)

The Foxen mudstone crops out only along the north flank of the. Casmalia Hills and extends beneath the Santa Maria Valley, where it attains a maximum thickness of about 3,000 feet near Betteravia. It laps upon the Franciscan and Knoxville(?) formations beneath the valley floor and does, not crop out along the north side'of the valley. The Foxen thins rapidly to the east and is missing beneath most of the Sisquoc valley.

The Foxen mudstone of this report corresponds to that designated by Woodring (Woodring, Bramlette, and Lohman, 1943, pp. 1353-1355). It includes only the mudstone, siltstone, and fine-grained $silty$ sandstone of middle(?) and upper Pliocene age, which rests conformably upon the Sisquoc formation in the western part of the area, and unconformably upon it in the eastern part. The fine-grained to medium-grained soft sandstone resting upon the siltstone has been considered a part of the Foxen by Frame (1938, pp. 30, 31) and Canfield (1939, pp. 54-60), but it is now distinguished as the Careaga sand.

WATER-BEARING PROPERTIES OF THE CONSOLIDATED ROCKS

The consolidated rocks are essentially not water bearing. Their denseness and high degree of compaction render them incapable of transmitting water. However, most· of the formations contain frao tures, joints, and fissures induced by folding and faulting. Conceivably such openings may convey small quantities of water to the adjacent unconsolidated deposits. A few wells have been drilled into see and the horth of at work at a the consolidated rocks in search of water for domestic and stock use,'

,particularly near the town. of Nipomo, but it: is reported that water encountered was of insufficient quantity to meet even these uses. However, some of the small springs in ravines along the south flank of the San Rafael Mountains issue from fractures in the older rocks. Therefore, it is believed that a relatively small quantity of water is transmitted to the main water body through such fractures in the .consolidated rocks. . .

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UNCONSOLIDATED WATER-BEARING DEPOSITS OF TERTLARY AGE CAREAGA SAND (PLIOCENE)'

Areal extent.-The Oareaga sand crops out along the north flank of the Casmalia and Solomon Hills, extends northward beneath the valleys of the Santa Maria and the Sisquoc Rivers, and laps upon the .consolidated rocks beneath the northern edge of the valley floors. (See geologic sections $A-A'$ and $C-C'$, pl. 2.) An isolated outcrop of tar-impregnated Careaga forms the north end of Fugler Point (pl. 1). *Stratigraphy.*-The Careaga sand, which is upper Pliocene in age, was formerly considered to be the uppermost member of the Foxen formation (p. 26), but is now generally distinguished as a separate formation. The Careaga rests conformably upon the Foxen mudstone in the central part of the Santa Maria Valley. Eastward, it laps unconformably upon the Sisquoc formation.

In most water wells the Oareaga is logged as sand-rarely as sand- ,stone, although in surface exposures it appears somewhat consoli- .dated. The induration is apparently just a surface featme presumably due to cementation, and does not extend to any appreciable .depth. Therefore, the name Oarega sand is used in this report rather than Careaga sandstone, as the formation has been described by Woodring.

Woodring (Woodring; Bramlette, and Lohman, 1943, pp. 1355-1356) recognized two members of the Careaga, which he distinguished as the Cebada fine-grained member and the overlying Graciosa coarsegrained member. For the purpose of this report they are treated as a single unit, which is shown on the geologic map.

Lithology and thickness.—The Careaga sand is composed primarily .of white to yellowish-brown loosely consolidated massive mediumgrained to fine-grained sand with some silt and with numerous lenses or "reefs" of megafossils. It is predominantly of marine origin. In the upper part it contains some lenses of soft conglomerate, the pebbles of which are well rounded and composed primarily of porcelaneous shale.

The maximum thickness of the sand is about 650 feet and occurs along the axis of the Santa Maria Valley syncline. Locally beneath the Horth Flank^o of the valley it thins to a minimum thickness of about 20

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feet. Most oil wells in the Santa Maria Valley oil field pierce the Oareaga, but in most of the area its top is several hundred feet below the depths penetrated by water wells (pI: 2). Along the north edge of the Santa Maria Valley a few wells penetrate the. Oareaga. The geologic section on plate 3 shows the position of the Oareaga beneath the eastern part of this valley. Wells 10/33~18H1, 10/33-18H2, 10/33-21R1, and $10/33$ -27D1⁴ are the only water wells that are known to have been drilled through the Careaga, which locally ratiges in thickness from as little as 20 feet to 120 feet. (For complete logs of representative wells see table 16.)

vVater~bearing *properties.-The* Oareaga sand bears the distinction of being the oldest water-bearing formation in the area, but it is probably one of the least permeable, owing principally to the contained silt. The loosely consolidated sand is capable of transmitting water through the openings or pore spaces between the grain particles. However, because the overlying formations are more permeable and because the· loose sand tends to "sand up" the wells, drillers do not perforate well casings in the Careaga sand. Although in this area its water-yielding capacity remains unknown, in the Santa Ynez basin yields of 150 gallons a minute or more have been obtained from the Oareaga by use of gravel-envelope wells. Presumably yields of this magnitude could be obtained in the Santa Maria Valley area.

Laboratory tests of permeability made on samples of the Oareaga· sand in the Santa Ynez basin (Upson and Thomasson, 1951), whete its lithologic properties are believed to be -essentially the same as in the Santa Maria Valley area, showed coefficients of permeability which averaged about 70 gallons a day per square foot at 60° F (Wenzel, 1942, pp. 7-10). When compared with that of the alluvium this permeability is quite low.

'PASO ROBLES FORMATION (PLIOOENE AND PLEISTOOENE1)

AREAL EXTENT

Like the Careaga sand, the Paso Robles formation crops out along the north flank of the Casmalia and Solomon Hills, it is folded downward in the synclinal trough of the Santa Maria and Sisquoc valleys, and the upper part is truncated on the north limb by younger deposits . (pIs, 1 and 2). Several minor isolated outcrops of the Paso Robles are found on the north side of the area. '

STRATIGRAPHY

The Paso Robles formation,⁶ which is upper Pliocene to lower Pleistocene (?) in age, was considered a part of the Fernando formation **Copy of document found at was expected**
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, For description of the well·numbering. system see p. 163. , Sometimes "designated the Schumann formation,"

by Arnold and Anderson $(1907, pp. 52-60)$. The Fernando formation. however, included all unconsolidated and some consolidated deposits from upper Miocene to lower Pleistocene. Both Frame (1938, pp. $28, 30$ and Canfield (1939, pp, 52-54) limited the Paso Robles formation to 200 to 500 feet of "blue gravels" resting upon the "Foxen sand," here called the Careaga sand, and underlying from 400 to 1,600 feet of stream gravels or "yellow gravels."

The Paso Robles formation as used in this report includes both the "blue" and "yellow gravels," as differentiated by Frame and Oanfield, and so conforms with the more recent work done by Woodring (Woodring, Bramlette, and Lohman, 1943, 1358-1359). The formation lies conformably upon the Careaga sand except locally near some valley. margins where it overlaps' the Careaga, and extends unconformably over the older Tertiary rocks, notably west of Tepusquet Creek. It
is overlain unconformably at one place or another by all the younger deposits.

. A deposit of massive fine white sand, over 125 feet thick and probably of marine origin, occurs along the axis of the Santa Maria syncline near Orcutt. This body has been observed only in water well logs. The sand is apparently overlain unconformably by the Orcutt formation (pI. 3), and may lie unconformably upon the Paso Robles formation. It may be a hitherto. unrecognized and distinct stratigraphic unit older than the Orcutt formation and younger than 'the Paso Robles. However, because its relatibn to the Paso Robles remains uncertain and, further, because the sand is of limited extent. it is tentatively assigned to the Paso Robles formation.

LITHOLOGY AND THICKNESS

The Paso Robles formation is probably the oldest nonmarine deposit in the area. In general it is composed of stream-laid lenticular beds or lenses of coarse to fine gravel and clay, medium to fine sand and · clay, silt, clay, and some lenses of gravel and sand. In· the lower part discontinuous thin limestone beds occur. However, the fact that the deposits on the south limb of the syncline appear to be somewhat finer-grained and of different composition than those forming the north limb, suggests a separate source for each and an inferred , .overlapping along the axis. .

The lithologic character and textural irregularity of the formation along the south side of the area are perhaps best shown by two relatively complete sections observed in the Santa Maria Valley and one partial section observed in the Sisquoc valley.

GEOLOGY AND GROUND-WATER, SANTA MARIA VALLEY, CALIF.

Section of about the lower three-fourths of the Paso Robles formation, exposed in ravine in the E_{22}^{1} sec. 34, T. 10 N., R. 35 W.

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ravine in the W $\frac{1}{2}$ sec. 15, T, 9 N, R, S4 W. Gravel, coarse, brown sandstone, porcelaneous shale, metavolcanics; sand, and some clay Gravel, medium; sand, and clay Concealed Sand, massive, medium, brown-Sand and coarse gravel showing cross bedding............................. Sand, coarse; and pebbles of brown sandstone, porcelaneous shale, and Clay, sandy to silty, $brown_{\text{max}}$, \ldots Sand, clay, and cobble gravel as large as 4 inches......................... Sand, brown; and cobble gravel Cobbles of porcelaneous shale and weathered-brown sandstone, rounded. Sand, massive, fine, clean, buff-Pebbles and cobbles of porcelaneous shale as large as 3 inches, and clay_ Sand, clayey, medium, gray; and pebbles of porcelaneous shale Sand, fine, clean, gray Pebbles, porcelaneous shale, maximum 2 inches; and gray clay

Concealed, probably clay, sand, and pebbles.............................

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Section of about the lower three-fourths of the Paso Robles formation, exposed in

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Section of about the lower three-fourths of the Paso Robles formation, exposed in ravine in the W1/2 sec. 15, T. 9 N., R. 34 W. - Continued

Section of part of the Paso Robles formation, exposed in Cat Canyon in the SEYSEY sec. 13, T. 9 N., R. 33 W.

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The lithologic character of the Paso Robles formation along the north limb of the syncline is known primarily from logs of wells. Beneath the Santa Maria and Sisquoc plains and the Orcutt upland water wells penetrate the Paso Robles formation for distances of from several feet to over 700 feet. Only those along the north edge of the Santa Maria plain pass through the formation, which in this area is represented by a truncated section. (See section $C-C'$, pl. 2.) The logs show that, except for a coarse basal gravel 10 to 30 feet in thickness encountered only by oil wells in the Santa Maria Valley oil field.⁶ there is no correlation possible between beds from place to place in the formation, and that the deposits are lenticular. However, the logs show that in general the Paso Robles contains large quantities of boulders and grawel, chiefly in a matrix of clay but locally including some sand. Westward near the coast the formation is composed mostly of sand and clay, which locally may be of marine origin, and some gravel and few boulders. (See logs for wells 10/35-7G1, and 17D1) and 11/35-19E1, 20E1, and 29R1, table 16.)

This formation forms the thickest single water-bearing deposit in the area. Geologic section $C-C'$ (pl. 2) shows that the formation reaches a thickness of about 2,000 feet near the town of Orcutt. This is believed to be the thickest section in the anea of Khaewhenendha

⁶ Dolman, S. G., personal communication, 1946,

thickness ranges widely. Water well 9/34-3N4 (table 16), which is the deepest well in the area (900 feet), is situated almost on the axis of this trough and penetrates the formation for a thickness of 716 feet-only about one-third the total thickness at this point.

WATER-BEARING PROPERTIES

The coarse-grained lenses of the Paso Robles formation supply a considerable quantity of water to wells. but the finer-grained lenses probably supply very little. As a whole the formation is a good waterbearing deposit, probably about as productive as the Orcutt formation, but considerably less than the alluvium. Few wells have been perforated in the Paso Robles alone, but those show that the formation is capable of yielding water to wells at rates as great as 1,000 gallons per minute. However, to obtain this high production the casings are perforated throughout a considerable section of the formation, and the wells have relatively low specific capacity, ranging from 5 to 10 gallons a minute per foot of drawdown.

The permeability of the formation has been determined by a recovery test (Wenzel, 1942, 125-129) in one pumped well near the coast where the deposit is generally fine-grained. The test was run on well 11/35-20E1, which penetrates only a part of the Paso Robles but the results of which are believed to be representative of the formation in that area. They indicate that the deposits tested have an average permeability of about 65 gallons a day per square foot, or about the same magnitude as that obtained for the Careaga sand (p. 28).

It can be concluded that the grain size and probably the watervielding capacity of the formation decreases toward the coast and from north to south. The numerous irrigation wells on the Santa Maria plain, therefore, probably tap the most productive part of the formation.

WATER-BEARING DEPOSITS OF QUATERNARY AGE ORCUTT FORMATION (PLEISTOCENE)

Areal extent. The Orcutt formation extends along the south side of the Santa Maria and Sisquoc valleys, and is believed to be present along the north side of the Santa Maria River (pl. 1). It does not extend beneath the alluvium in the Sisquoc valley nor beneath the greater part of the Santa Maria Valley (pl. 2). However, beneath most of the Oso Flaco district (p. 19) it may be present where the lower part of the alluvium is absent (pl. 3).

Stratigraphy. The Orcutt formation is an essentially nonmarine. slightly deformed, relatively thin deposit of upper Pleistocene age, New MetaTrests unconformably primarily upon the Paso Robles formation-the degree of unconformity becoming more pronounced on

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limbs of folds. Locally it rests unconformably upon the older rocks. According to Woodring (Woodring, Bramlette and Lohman, 1943, p. 1359), the type region is on the north flank of the Casmalia Hills west of Orcutt, where it attains a thickness of about 50 feet, and is primarily sand. However, the logs of wells indicate that only a small section of the Orcutt is represented in this locality, and that along the axis of the Santa Maria syncline it locally attains a maximum thickness of about 225 feet. The am train in the street of the in the America

Furthermore, the logs indicate that the formation is composed of two conformable members-an upper fine-grained sand member which corresponds to that portion of the formation exposed at the type locality, and a lower coarse-grained member. Because the two members differ lithologically, the Orcutt in this report has been designated a formation rather than the "Orcutt sand," as named by Woodring. Woodring's term seems to apply only to the upper member. Parts of both members have been observed in exposures, and they are shown on plate 3. However, they are not distinguished on the geologic $\text{map}(pl. 1).$

In addition, the uppermost and partly deformed "terrace deposits" mapped by Woodring south of the Sisquoc valley are believed to be equivalent to the Orcutt formation, because of their physiographic and stratigraphic position and structural features. They have been assigned to the Orcutt formation in this report (pls. 1 and 2). Dips as great as 12° have been observed in the formation along the north flank of the Solomon Hills, which is perhaps unusual for deposits of upper Pleistocene age.

Lithology and thickness.—Because the entire formation cannot be observed at any one exposure in the area, the study of the lithology is based necessarily on both surface and well-log data. The upper member is mostly a loosely compacted massive medium-grained clean sand, stained reddish-brown by a ferruginous cement and interstratified with lenses of clay. Locally the sand beds themselves contain clay. Nearthe north edge of the Orcutt upland the upper member contains lenses of gravel (pl. 3). Where exposed the member usually stands in nearly vertical cliffs. It ranges in thickness from a feather edge to about 225 feet along the axis of the Santa Maria syncline.

The lower member is chiefly loosely compacted, coarse gray to white gravel and sand. Its contact with the upper member is sharp, and in surface exposures the lower member is usually intricately rilled and fluted. It is quite difficult to distinguish from the underlying Paso Robles formation, particularly where the unconformity between them is slight. It ranges in thickness from a feather edge to 65 feet, fund at

Like the Paso Robles formation, the Orcutt is fine-grained near the coast, and well logs indicate that the deposits there are predominantly sand and clay throughout and in part may be of marine origin. In the Sisquoc valley the Orcutt is coarser in grain, and distinction between its two members is impossible.

Water-bearing properties.—The Orcutt formation supplies water to wells in appreciable quantities only beneath the Orcutt upland, where the lower member is one of the principal water-producing deposits. It supplies water of perhaps the best quality in the area to the city of Santa Maria and the town of Orcutt. These municipal wells are in secs. 3 and 10, T. 9 N., R. 34 W. However, in the years 1938-42 water levels in these wells fell below the top of the lower member; since then they have recovered (well $9/32-3N3$, fig. 5). Toward and beyond the eastern end of the upland the member rises above the water table and is therefore useless as a source of supply. To the west it becomes less productive, until at the coast it is composed mostly of clay, silt, and fine sand, and is there considered a poor water-yielding deposit.

No tests of permeability have been made on either member of the Orcutt, but where the public-supply wells draw on the lower member, its permeability is probably considerably greater than that of the underlying Paso Robles. Also, because of its lithologic characteristics, the lower member here is probably considerably more productive than the upper member. Elsewhere the wide range in lithology obviously is accompanied by a corresponding range in productivity.

TERRACE DEPOSITS (PLEISTOCENE)

Areal extent.-Terrace deposits compose and underlie the 40-foot and 100-foot terrace surfaces previously described (p. 20). They are remnants of more extensive deposits but even now underlie the greater part of the Orcutt upland, the Nipomo upland, and numerous smaller areas along the Sisquoc and Santa Maria Rivers (pl. 1).

Stratigraphy and thickness. The terrace deposits are the alluvial materials that were laid down by streams during the formation of the 40-foot and 100-foot terraces. They rest unconformably on the Orcutt formation, and locally on all older formations, and are in turn overlain locally by dune sand. They are older than the alluvium and are considered to be upper Pleistocene in age. They range in thickness from a feather edge to a maximum of at least 45 feet. (See log for well $9/32-7A1$, table 16.) Beneath the extensive surface of the 40-foot terrace on the Orcutt upland the deposits are a thin veneer roughly 5 to 10 feet thick; but they are considerably thicker immediately south of Fugler Point where they fill an old channel.

Lithology.—The terrace deposits are composed essentially of unconsolidated boulders, gravel, sand, silt, and clay intermixed to varying degrees and occurring in poorly defined lenses. The coarse-grained

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portions are buff-colored. In general, the deposits are similar to the coarse-grained parts of the alluvium.

Water-bearing properties.—Near the rivers, and where they overlie unconsolidated deposits, the terrace deposits are mostly above the. zone of saturation and hence supply little water to wells. However, near and southeast of Nipomo, the deposits rest on consolidated rock. and there contain water in the lower part in quantities sufficient to meet domestic and stock requirements. The deposits are coarsegrained and porous, and hence readily absorb rain which they transmit to any underlying permeable formations.

ALLUVIUM (RECENT)

The alluvium, which is the most productive water-bearing deposit in the area, is unique in that it is almost completely concealed by its own. surface. Because of this the extent, stratigraphy, thickness, lithology, and water-bearing properties of the alluvium all were determined entirely from well logs and pump tests. Over 350 water-well logs and numerous oil-test holes that pierce the alluvium were studied in detail. This study was considerably aided by a peg model, which presented a. three-dimensional picture of all available well logs.

Stratigraphy.-The alluvium, as the name implies, is a body of alluvial deposits laid down by streams graded initially to a position of sea level about 230 feet below present level. It is believed to have been deposited as sea level rose during the retreat of the Wisconsin ice sheet, and is therefore considered to be Recent in age. The alluvium comprises two members-an upper fine-grained member, and a lower coarse-grained member. It is unconformable on all older deposits, but throughout the area rests chiefly on the Paso Robles formation (pl. 2). It is itself locally overlain by river-channel and dune sand deposits.

The stratigraphic units and position, physiographic expression, lithologic character, and thickness of the alluvium in the Santa Maria. Valley area correspond to those of the alluvium found in other coastal valleys of southern California (Fairbanks, 1904, p. 13; Poland and Piper, in preparation; Upson, in preparation; Upson and Thomasson, in preparation.)

Areal extent.-The upper member of the alluvium underlies and forms the surfaces of the Santa Maria and Sisquoc plains, and the alluvial plains of tributary streams (pl. 1). It also extends beneath the channel deposits of all the major rivers and streams. The lower member has essentially the same extent as the upper member, with one major exception. It is missing beneath that portion of the Oso Flaco district lying roughly north of latitude 35°00'. (See pls. 1 and 2.)

Thickness.-In logs of wells the base of the colly vivoruse meadily recognized beneath the Sisquoc River near La Brea Creek, and beneath

the Santa Maria and Cuvama Rivers near Fugler Point where the deposit rests on consolidated rocks or the Careaga sand; but it is not easily recognized over the greater part of the area where it rests on the Paso Robles formation. However, by comparing logs where the base is doubtful with logs of nearby wells in which it can be recognized, and by projecting the slope of consolidated rock surfaces overlain by alluvium, the base can be fairly accurately determined everywhere. As thus determined the alluvium ranges in thickness laterally from a feather edge at the north and south margins of the alluvial plains to maximum thicknesses beneath the central parts. These maximum thicknesses range from 50 feet at the upper end of the Sisquoc plain to 115 feet at Fugler Point (an average increase in thickness of 8 feet per mile in this reach); and to 230 feet at the coast (an average increase in thickness of 6 feet per mile for the reach below Fugler Point). Thus, the deposit thickens almost uniformly westward beneath the alluvial plains. (See pl. 3.)

At the coast the two members are each about 115 feet thick, and each thins eastward. However, the lower member thins more rapidly and near Fugler Point is about 40 feet thick, whereas the upper member there is about 75 feet thick.

Lithology.-The detailed lithologic character of the alluvium is shown by the logs of representative wells in table 16, and the two members are distinguished whenever possible. The logs show that the lower member of the alluvium is composed primarily of boulders, gravel, and sand, with minor lenses of clay interfingered near the coast. The basal part of the lower member is particularly coarse. and is usually denoted by well drillers simply as boulders, or gravel. or both. In general the grain size decreases slightly as the deposit thickens toward the coast.

The lithology of the upper member, like that of the lower member. is known primarily from logs of wells. Beneath the Sisquoc plain the upper member is practically indistinguishable from the lower member—both being composed of boulders and gravel and some sand. Beneath the eastern and central parts of the Santa Maria plain, the coarse gravel and boulders of the lower member are overlain by sand and gravel or sand in the upper member, and the contact between the two is distinguishable. From the city of Santa Maria to a point about halfway to Guadalupe, the sand and gravel of the upper mem- . ber grade rapidly to sand and silt with progressively fewer beds of gravel. From this point westward to the coast it is composed of alternating beds predominately of clay and silt with some sand and a few gravel layers. Thus, the upper member decreases rapidly in Nemminipsiere domom east to west.

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 \leq Near the coast the contact between the upper and lower members is sharp and is easily identified in well logs. The individual clay beds; which are compact and usually reported as blue, are relatively extensiye, especially those commonly encountered near the surface. However, from the data at hand it cannot be definitely concluded that individual clay beds extend as one continuous unit entirely across the west end of the valley. It is thought that some of these claybeds are of marine origin and were deposited at times when the rise of sea level was faster than the accumulation of fluvial debris. Other clay beds, reported as yellow in drillers' logs, are possibly of fluvial origin, their color presumably resulting from surface exposure and oxidation of contained iron compounds. From one place to another the clay beds range in thickness from 1 foot to about 100 feet--.almost the full thickness of the member. .

Water-bearing properties.—The lower member of the alluvium, which at present is completely satuxated, yields 'water readily to wells. For example, wells 10/33-21R1 and 10/33-36A1, which derive water solely from the alluvium, each have a yield of about 1,000 gallons a minute and a drawdown of about 45 feet; or a specific capacity of about 22 gallons a minute per foot of drawdown.

The upper member, on the other hand, has a wide range in ability to transmit and to yield water. In the eastern part of the area, where the deposits are similar to the lower member, the yield is high; but at the west end of the valley the fine-grained sediments, although saturated, are essentially not water yielding and are capable of transmitting water to wells only in small or negligible quantities. In the intervening area the yield is gradational. The fine-grained and irregular beds which compose the upper member at the west end of the area form a seal of varying tightness due to overlapping of one lens upon the other and there confine the water in the underlying deposits. (See pp, 72-73.) Hower member of the alluvium, which and down the

elower member of the alluvium, which down the

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. Tests of permeability of the alluvium by use of the recovery method in a pumped well (Wenzel, 1943, pp. 125-129) were made on wells $f(32-24K1 \text{ and } 10/33-21R1.$ (pl. 1). In both wells the alluvium rests on consolidated rocks. Results obtained from these tests showed permeability coefficients of 4,500 gallons a day per square foot for well $24K1$ in the upper Sisquoc valley, and 3,500 gallons a day per square foot for well ZlRl in the upper Santa Maria valley. This indicates that the permeability of the alluvium is high and that it decreases in a .downstream direction as the material becomes somewhat finer-grained.,

Similar tests were run near the coast on wells 10/35-5J1, 8Q1, 17D1, $,11/35-29D1,$ and $32R1$, which are perforated in the lower member of the alluvium and in the upper part of the underlying less permeable, we have newwiptax. Paso Robles formation. The average of the five tests, which in

themselves were not entirely satisfactory owing to irregularities in the · recovery curves, showed the composite permeability of both formations to be about 1,500 gallons a day per square foot. Obviously then, the permeability of the lower member of the alluvium alone is somewhat ereater than 1,500. It has been indicated that the permeability of the alluvium at well $10/33-21R1$ was 3,500 gallons a day per square foot. and that the permeability probably continues to decrease westward as the deposits become finer-grained. Hence, the permeability of the alluvium near the coast is probably considerably less than 3,500 gallons a day per square foot, but somewhat greater than 1,500. A coefficient of about 2,000 gallons a day per square foot is 'considered to be of the correct order of magnitude.

RIVER-CHANNEL DEPOSITS (REOENT)

Areal extent.-The river-channel deposits extend some 30 miles down the full length of the Sisquoc and Santa Maria plains. Along the Sisquoc plain and the upstream half of the Santa Maria plain they fringe the north edge of the plains, but downstream they cut diagonally across the plain to the southwest corner. In the lower course of Cuyama River the channel deposits occupy most of the surface area of the canyon floor (pl. 2).

Stratigraphy and thickness.—The river-channel deposits consist of the gravel, sand, and silt contained within the banks of the major rivers; these deposits extend downward to and rest unconformably 'upon the upper member of the alluvium, Because few wells are drilled.. in the channel deposits and because of the similarity between these deposits and the underlying alluvium, the maximum thickness is not definitely known but is not believed to exceed 25 feet.

Lithology.-The lithology of the channel deposits is known only from surface examination. In general these deposits are extremely coarse-grained in the Cuyama and Sisquoc River channels and relatively fine-grained in the lower reaches of the Santa Maria River. channel. The deposits of the Sisquoc River channel are composed of boulders, gravel, and coarse sand intermixed in bars or lenses of varying coarseness. The boulders attain a maximum size of over 1 foot in the coarseness. The bounders attain a maximum size of over 1 foot in diameter, but more commonly are smaller. The coarser constituents are composed primarily of hard sandstone and of metavolcanic rocks derived from the hea are composed primarily of hard sandstone and of metavolcanic rocks

"The deposits of the Cuyama River contain considerable silt which is derived from massive silt beds that crop out in the Cuyama Valley (pl. 4), intermixed with the coarser material. During high water and during the following-recession the silt gives the water an orange color and A-soupy appearance. Even during low flow considerable silt is. carried by the river.

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The deposits of the Santa Maria River channel are necessarily a combination of the materials carried by the Cuyama and Sisquoc Rivers. At Fugler Point, the deposits consist of coarse sand and silt with numerous pebbles. Westward the material becomes progressively finer, and near Guadalupe medium to fine sand and some silt with occasional pebbles form the main body of the deposits.

Water-bearing properties. The water-bearing properties of the channel deposits are of particular importance because they transmit the large seepage losses that occur throughout the Sisquoc and the greater part of the Santa Maria channels whenever there is any runoff. Except along the Cuyama River, and possibly for some distance along the Sisquoc River below La Brea Creek, the major part of the channel deposits lie above the water table, and hence, transmit the seepage losses vertically downward.

TABLE 2.-Results of permeability tests on samples of river-channel deposits in the Santa Maria, Cuyama, and Sisquoc Rivers

Santa Maria River channel

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The water-bearing properties of the unconsolidated and relatively coarse-grained channel deposits are perhaps best indicated by laboratory tests of permeability that were run on samples collected along the courses of the Santa Maria, Sisquoc, and Cuyama Rivers. The permeability coefficients were obtained by use of a variable-head appratus designed by S. F. Turner, United States Geological Survey, and similar to that described by Wenzel (1942, pp. 59-65). Two samples were taken on opposite sides of the active channel at 12 locations and the average permeability for each location is shown in table 2.

The table shows that the permeability of the deposits increases upstream and reaches a maximum value of 1,060 gallons a day per square foot in the upper part of the Sisquoc valley.

DUNE SAND (RECENT)

Areal extent.—The dune sand covers about 25 square miles of the Orcutt upland, about 15 square miles of the Nipomo upland, and about 10 square miles of the Santa Maria plain along the coast (pl. 1).

Stratigraphy, lithology, and thickness.—The dune sand deposits are Recent in age, and are found to be of two types: actively drifting dunes which are encroaching over the older deposits near the coast; and the old or inactive dunes which are anchored by vegetation and which in part have a well-developed soil mantle. They have not been differentiated on the geologic map. Both rest unconformably on older deposits. The dune sand is composed primarily of coarse to fine, well-rounded massive characteristically cross-bedded quartz sand. loosely to slightly compacted. The dunes range in thickness from a feather edge to more than 100 feet.

 \mathbb{W} *Water-bearing properties.* The dune sand lies above the surface of the main water body but contains several small perched or semiperched water bodies, which locally supply water to a few domestic wells in the Orcutt upland. Thus, the sands are known to transmit and to yield water. However, because no tests were made, the permeability of the sand is unknown.

GEOLOGIC STRUCTURE GENERAL REGIONAL STRUCTURE

The regional structure surrounding and including the Santa Maria Valley area is extremely complex for it lies within the structural influence of both the California Coast Ranges and the so-called transverse ranges of southern California. Physiographically and structurally the San Rafael Mountains lie at the southern edge of the

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OaJiforma Ooast Ranges; whereas the Santa Ynez Mountains to the south form the western part of the westward-trending transverse ranges (fig. 1). The region included between the two ranges is a structural depression, and the older rocks, which are exposed in the bordering ranges, here are concealed at considerable depth beneath Tertiary and Quaternary rocks. The tertiary rocks form a series of broad f01ds whose axes have a general westward trend.: Of these the northernmost downfold forms the basin beneath the Santa Maria and Sisquoc valleys. The shape and extent of this major syncline. and the faults which transect it, and their relation to the groundwater basin are discussed below.

. MA.JOR SYNCLINE

The major syncline that underlies the valley area is an asymmetric structural trough whose axis trends southeastward along the south side of the Santa Maria Valley, in the vicinity of Orcutt veers sharply northeastward toward Fugler Point, and finally turns southeastward near Garey into the Sisquoc valley. Its exact course and shape between Orcutt and Garey is not definitely known, and therefore, it is not shown on the geologic map $(pl. 2)$. The offsets or bends in the axis are probably due to the regional stresses that exist between the Coast Ranges and transverse ranges. The shape and lateral' extent of the syncline are shown on the geologic cross sections $(p1, 3)$.

The south limb of the syncline, which is steeply dipping beneath the Santa Maria Valley and gently dipping beneath the Sisquoc valley, forms the north limb of the major anticlinal structure beneath the Casmalia and Solomon Hills. Minor en échelon folds having a northwest trend are prominent features of the south limb. In the Sisquoc valley the north limb rises steeply to form the south flank of the San Rafael.Mountains, but in the Santa Maria.Valley it rises gently and is cut out by the alluvium.

FAULTS

In the bordering hills which are underlain by consolidated rock the faults were observed only casually and for the most part their locations are taken from work of other geologists (Woodring, Bramlette, Lohman and Bryson, 1944).⁷ In general these faults have a westward trend in the Solomon and Casmalia Hills and have a northwestward trend in the San Rafael Mountains. As such they bear little relation to the ground-water basin. However, several faults cut the waterbearing deposits of upper Pliocene and lower Pleistocene (?) age. namely, the Careaga sand and the Paso Robles formation. The faults Copy of document found at

 $'$ Also Greenwalt, W., personal communication.

are in a position to affect the movement of ground water in those formations. They are concealed by the younger. deposits and their existence has been determined primarily by studies of oil-well logs.

The faults are three in number and trend slightly west of north. It is thought that movement along all three is predominantly verticaL The fault extending southeastward from Santa Maria was encountered in oil wells drilled in the Santa Maria oil field; it has been plotted at the location shown by Canfield $(1939, p. 48, fig. 2)$. It is a high-angle thrust·fault and in this report is referred to as the Santa. Maria fault. Uplift has taken place on the east side, and the fault cuts all formations up through the Paso Robles. The maximum amount of displacement in the Careaga sand and Paso Robles formation is about 150 . feet, but displacement' in the older rocks increases with depth. (See Uplift has taken place on the east side, and the fault cuts all forms
Uplift has taken place on the east side, and the fault cuts all forms
tions up through the Paso Robles. The maximum amount of displace
ment in the Care

East of the Santa Maria fault and roughly beneath Bradley Canyon is the second of the three faults, which is herein named the Bradley Oanyon fault. The presence of this fault was determined primarily from oil-well logs, which indicate an offset in the older rocks. This, faulting in Bradley Canyon is presumed to extend beneath the plain into a small fault of the same general trend observed on the north side of the Santa Maria River. Like the Santa Maria fault, it cuts the Careaga sand and Paso Robles formation, but, unlike the Santa Maria fault, the west side is believed to be uplifted and the amount of displacement is somewhat less. The straight-line appearance, the direction, and the location of Bradley Canyon possibly reflect topographically the existence of the fault.

'A third fault having the same trend and age as the other two may cross the upper end of the Santa Maria Valley at Fugler Point. Because. its existence is doubtful and because its location 1s uncertain, it is not shown on the geologic map nor on the cross sections. The existence of the fault was first suspected in the preliminary study of water-level contour maps, which show a sharp break in hydraulic . gradient beneath the valley floor west of Garey (p1.5) . However, later studies show that the break could be caused equally well by other conditions $(p. 75)$. Additional inconclusive evidence was the presence of small tar seeps in the Careaga sand at Fugler Point, suggesting a fracture zone along which the tar might be rising. The most likely evidence is a fault in the consolidated rocks on the north side of the river and trending generally toward the area in question. Considerably more evidence will be needed, however, before the presence and location of the fault can be established and its relation to the movement of ground water can be ascertained. .

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RELATION OF STRUCTURE TO THE GROUND-WATER BASIN

The major synclinal trough is the structure which has determined the shape of the ground-water basin, whereas the faults have altered its shape but slightly. The shape of the contact between the un-,consolidated water-bearing deposits and underlying consolidated rocks is an inherent part of the trough and, as such, limits the lateral and downward extent of the ground-water basin. Specifically, this contact as exposed on the northern and southern flanks of the Santa Maria and Sisquoc valleys marks the northern and southern limits or sides of the basin, and where the contact swings around the head of the Sisquoc plain it forms the eastern end. The concave upward surface of the contact forms the base or bottom of the basin.

On the other hand, at the west end of the valley the syncline, and' hence the contact, passes out to sea. As a'result the unconsolidated deposits and the contained water body extend out beneath and lie in contact with the Pacific Ocean. Thus there is no known structural or depositional barrier between the fresh water of the main water body and the salt water of the Pacific Ocean.

GEOLOGIC HISTORY

EARLY HISTORY

The early geologic history of the Santa Maria Valley area bears only an indirect relation to the present ground-water basin and the existing hydrologic problems, and it is therefore summarized very briefly. More complete accounts from which the summary has been drawn are presented principally by Woodring (Woodring, Bramlette; and Lohman;)943, pp.1338-1343) and Canfield (1939) pp, 79-81), and in the classic report by Arnold and Anderson (1907, pp. 66-71).

The erosional surface developed on the Jurassic rocks was submerged and covered by the sea with only minor fluctuations from late lowet Miocene until upper Pliocene time. Deposition in this sea began-with the accummulation of fine-grained materials composing the Monterey: shale, which was followed in turn by the Sisquoc and Foxen formations· . and ended with the deposition of the Careaga sand. This period of marine deposition was accompanied by continued uplift and folding' and ended with the deposition of the Careaga sand. This period of marine deposition was accompanied by continued uplift and folding along and near the present San Rafael Mountains.

With deposition of the Oareaga sand in upper Pliocene time, the basin was filled to sea level except along the axes of the synclinal troughs, which were still submerged. It was upon this surface that the continental Paso Robles formation was deposited. The lower and western parts of this formation, however, are locally of lagoonal or.

brackish-water origin because they were laid down in the still-submerged synclinal troughs. The deposition of the Paso Robles continued into the lower Pleistocene (?). The northern limit of Paso Robles deposition was probably the ancestral San Rafael Mountains, from which a considerable quantity of coarse material was derived; while on the south material of fine texture was probably derived from jupland areas far to the southeast of the present Santa Maria Valley area. Minor warping accompanied the deposition of both the Careaga sand and the Paso Robles formation, thus accounting for the presence of the thickest sections in the troughs of synclines and the thinnest :sections along the axes of anticlines.

HISTORY.OF THE GROUND-WATER BASIN

Structuralevoluti01i.-Following the deposition of the Paso Robles formation, intense folding took place probably during middle Pleistocene time *(Poland and Piper, in preparation)* along established structural lines, and the existing limits of the ground-water basin were established. The Careaga sand and Paso Robles formation were arched over the Casmalia and Solomon Hills, were depressed into the large synclinal trough, and were cut by faults. It is believed that during the same period the Franciscan and Knoxville(?) rocks along the north side of the area were further uplifted. Thus, in middle Pleistocene time the lateral and downward limits or shape of the basin were defined broadly as they now exist.

Relatively stable conditions followed the intense folding of the middle Pleistocene and persisted into the upper Pleistocene (Woodring, Bramlette and Lohman, 1943, p. 1342). During this relatively long interval of time, stream erosion developed a gently seaward-sloping surface roughly between the San Rafael and Santa Ynez Mountains. Deposition of the Orcutt formation took place on this surface in upper Pleistocene time. Erosional activity in the ancestral headwater areas was probably vigorous at first and the coarse-grained lower member was deposited. Less active conditions prevailed during the deposition of the fine prained upper member. Local coastal submergence is believed, to account for the presence of the interfingered marine beds .in the western extent of the Orcutt formation.

.' Folding and local minor faulting took place along the. developed structural lines following the deposition of the Orcutt formation, but prior to that of the late Pleistocene terrace deposits. Thus, the post-Orcutt deformation marked the final phase in the structural evolution of the ground-water basin.

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Erosional and depositional evolution.-The subsequent development of the basin took place almost entirely through erosion and deposition by streams in late Pleistocene and Recent time.' It is believed that the ancestral rivers and streams were located approximately at their present positions and were developed on the surface of the deformed Orcutt formation. Thus, the Santa Maria and lower Sisquoc Rivers. are essentially consequent and are situated in the structural trough: formed between the Solomon and Casmalia Hills on the south and the San Rafael Mountains on the north. The courses of the lower Cuyama. and upper Sisquoc Rivers, however, are antecedent, and transected the axis of the San Rafael Mountains at an earlier time.

The ancestral streams are believed to have cut the terrace floors and to have placed the deposits whose surfaces now remain at elevations of about 100 feet and 40 feet above the present river courses. The 100 foot terrace, which is the older of the two, was probably formed during a period of relative stability as the ancestral rivers were cutting down through the surface of the deformed Orcutt formation.

In general the history of the 40-foot tenace is fairly well preserved in the outcrops adjacent to the present channel courses. Following the formation of the 100-foot terrace the ancestral rivers cut down at least 100 feet, and possibly as much as 135 feet, below that surface probably in response to a lowering of sea level. (See log for well 9/32-7 AI, table 16.) Their .entrenched valleys occupied the fuil width of the present Sisquoc plain plus the terrace surface to the north,' passed south of Fugler Point, and probably followed a course westward down the central part of the present Santa Maria plain to the coast. A: subsequent rise in sea level of at least 40 feet, and possibly as much as 75 feet, caused the ancestral rivers to backfill their excavated courses to a height of about 40 feet above the present alluvial plains.

A period of relative quiescence followed the deposition, during which

the rivers cut laterally into the adjacent deposits. During this time, the existing relatively extensive cut terrace was formed on the Orcutt upland, and the river cut northward into the consolidated rocks on the north side of Fugler Point. In geologic time this period may correspond to the interglacial period prior to the advance of the Wisconsin glacial sheet.

Sea level again began to decline, possibly coincident with the advance of the Wisconsin glacial sheet, and the rivers again began to down cut. This down cutting took place principally along the same course taken by the rivers during the previous down cutting, with one notable exception. The Santa Maria River, instead of reexcavating its channel south of Fugler Point, became established north of Fugler .Point, about on its present comse. .

Down cutting continued until the rivers were graded to a sea level possibly as much as 300 feet below the present sea level and several miles west of the present shore line. During this process, terrace deposits were almost completely removed, and only small remnants now remain along the sides of plains and river channels (pl. 1). At the present coast the down cutting amounted to about 230 feet below current river grade, and at the eastern end of the Sisquoc valley the down cutting amounted to about 50 feet. (See geologic sections $E-F$, $R-G$, and $G-H$, pl. 3.) The trench thus excavated was a relatively flat featureless plain of about the same extent as the present alluvial plains, had a steeper surface gradient than the present plain (p. 37), and had one relatively large bench or terrace in the Oso Flaco district above the excavated floor, at a height about midway between the present alluvial plain and the bottom of the excavated trench (p. 36), or about .100 feet below present land surface. (See logs for wells 11/35-20E1 and 11/35-27H1, table 16, and pl. 3.) Although in this area there is no definite proof that this bench was formed as the. river was down cutting, Poland (Poland and Piper, in preparation) has been able to show that the formation of similar terraces occurred during the down cutting in the vicinity of Long Beach, California. . Deposition in. the excavated trough began and continued as long as sea level rose. Again the rise may be coincident with the retreat of a glacial ice sheet. If so and, further, if the ice sheet was the last or Wisconsin glacial sheet, then the initial deposit formed in the bottom of the trough marks the beginning of the Recent epoch. It has been (estimated by Schuchert. and Dunbar (1933, p. 479) that the retreat of this ice sheet, and hence the initial deposition, may have begun approximately $27,000$ years ago.

The deposit formed during the initial stages. was. the lower member !of the alluvium. Its coarseness can be attributed to vigorous erosional activity in the headwater areas caused by exceedingly wet climatic conditions, a large volume of river discharge, which transported considerable quantities of coarse material into and through the area, and an average land-surface gradient of about 24 feet per mile, compared to the present average of about 18 feet per mile, or about 30 percent steeper than the present gradient. Deposition of the coarse material comprising the lower member continued until it attained a maximum thickness of about 115 feet at the present coast line (pl. 3).

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SURFACE-WATER RESOURCES

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Following the deposition of the lower member, drier climatic conditions apparently prevailed and caused an abrupt decrease in erosional activity in the headwater areas and the deposition of the fine-grained]. sediments of the upper member within the plains.

The abrupt change in depositional activity is indicated by the sharp contact between the two members of the alluvium near the coast, as shown on cross sections $E-F$ and $I-I'$ (pl. 3). Deposition: of the upper member by the ancestral Santa Maria River at times: took place more slowly than the rise of sea level. Consequently, brackish water or lagoonal clays and beach sands are interfingered. with the fluvial deposits near the coast.

Guadalupe Lake, which has a depth of as much as 25 feet, probably· owes its existence to the fact that the alluvium was deposited at \boldsymbol{x} : more rapid rate by the Santa Maria River than by the creek entering' the plain through the lake from the southeast. Oonsequently, a, closed basin was formed in the lower course of the creek.

The Sisquoc and Santa Maria plains·now.form the surface of the . upper member of the alluvium, and the present channel deposits of the Sisquoc and Santa Maria Rivers have been deposited on that, surface. The Sisquoc River and the upper part of the Santa Maria, River have maintained courses along the north side of the alluvial plains throughout historic time. The present relatively stable position. .of the channels is caused largely by man-made control in the form of. jetties, which are built out into the river channels.

The sand of the relatively large area of dunes on the surface of the alluvial plain and adjacent upland areas has been brought along the shore of the Santa Maria Valley by waves and longshore currents: from the headlands projecting into the Pacific Ocean northwest of San Luis Obispo. The prevailing northwest winds, occasionally of gale velocity, have blown the sand inland and are continuing to do so; The extent of the dunes on the plain is limited in part by the action of the Santa Maria River.

SURFACE-WATER RESOURCES

By H. G. THOMASSON, JR.

The over-all drainage system of the Santa Maria River basin encompasses about 1,800 square miles. This system embraces the drainage basins of two major rivers-the Cuyama and Sisquoc and their tributaries, an area of about 1,600 square miles---all above their confiuence at Fugler Point; also, the drainage basin of the Santa River proper, about 200 square miles, downstream from Fugler Point.

This drainage system is here divided into a mountainous headwaterarea underlain at shallow depth by older consolidated rocks, and a downstream segment or valley area underlain to substantial depth by unconsolidated and largely permeable deposits. The headwaterarea includes all of the 1,600 square miles of the drainage basins upstream from Fugler Point, except the Sisquoc plain and a part of the dissected upland to the south, as shown within the limits of plate 2. Thus, it is almost wholly outside the area for which the geology and . ground-water conditions .are appraised in this report. .

In this treatment of surface-water resources, all stream flow is: considered as originating in the headwater area and, because surface runoff from the valley area is relatively small, its contribution is: included in the evaluation of rainfall infiltration $(p, 80)$. The geographic distribution and extent of the several drainage basins are shown on plate 4.

The Cuyama and Sisquoc Rivers deliver large quantities of runoff to the valley area. In times of flood, much of this runoff is wasted to the ocean; during periods of low or moderate flow, all or most of the water entering the area is absorbed by the river-channel deposits and is contributed as recharge to the ground-water supply. Thus, determination of the total-runoff from the headwater area, and of the seepage losses occurring within the valley area, is necessary in order to evaluate the natural recharge to ground water in the valley area. Accordingly, in the ensuing pages, data on surface-water resources. are presented to show the estimated total amount and distribution of runoff in the Ouyama and Sisquoc Rivers .and their tributaries at, about the edge of the Santa Maria Valley area, the estimated amount of natural seepage loss that takes place from these rivers and from the Santa Maria River within the valley area of ground-water recharge, and the estimated amount of surface-water outflow to the sea.

GENERAL CHARACTERISTICS OF RUNOFF

Because the 1,600 square miles of the headwater area includes: terrain ranging from the relatively wet Sisquoc and Huasna River drainage basins to the semiarid Cuyama Valley, and further, because rainfall occurs largely in a few storms during a rainy season that, extends from about November to April, runoff varies considerably among the several stream drainage basins and fluctuates greatly from: wear to year. During the 16 years. 1930-45, for which gaging-station records are available for the Cuyama and Huasna Rivers, the greatest runoff in the Cuyama River was 21 times the least yearly

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runoff, the extremes occurring in 1940-41 and 1933-34, respectively: whereas in the Huasna River this ratio was 259, extremes occurring in 1940-41 and 1930-31, respectively. Maximum monthly measured discharge in the Cuyama River was 33,320 acre-feet (March 1938), and in the Huasna River was 24,150 acre-feet (February 1941). In years of low rainfall each of the streams has been observed to be dry. for periods of several months. These figures are rather remarkable considering that the drainage area above the Cuyama River gaging station is 912 square miles, whereas the area above the Huasna station. is only 119 square miles.

GAGING-STATION RECORDS AND SUMMARY OF STREAM FLOW

The following table indentifies the gaging statons at which continuous records of stream flow have been obtained, the periods of those records, and water-supply papers in which they have been published. As shown on plate 4, the gages record the runoff from practically all the drainage area tributary to the Santa Maria Valley area upstream from Fugler Point. Within the valley area the station at Guadalupe measures essentially all surface-water outflow from the valley. $\verb|value|$, which is a set of $\verb|value|$, $\verb|value|$, $\verb|value|$, $\verb|value|$, $\verb|value|$, $\verb|value|$

Available records of *stream flow in the Santa Maria River drainage system*

1 Records collected on Ouyama River at mouth of Buckhorn Canyon, 6.5 miles upstream from present
gaging station, Ouyama River near Santa Maria.

N oTE.-Records bere listed have been publisbed by the Geological Survey as follows:

il;

The gaging station on the upper Cuyama River hear Ozena (pl. 4) was installed in October 1944. Insufficient records are available at that site to be of use in this report ...

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Only two gaging stations have been operated continuously since 1930—namely, the station on the Cuyama River near Santa Maria. 2½ miles above Alamo Oreek, and the station on the Huasna River near Santa Maria, half a mile above the mouth, (See preceding table.) A gaging station was operated on the Sisquoc River near Sisquoc, about 21/2 miles above La Brea Creek, from December.1929 to September 1933; and at the same site since October 1943. The station on the Sisquoc River near Garey, about half a mile below the mouth of Tepusquet Creek and within the valley area, has been operated since February.1941. The stations near the mouths of Alamo, La Brea, and Tepusquet Creeks have been operated since October 1943. The station on the Santa Maria River at Guadalupe, which measures surface water leaving the valley, has been operated since January 1941. 1930—namely, the station on the Cuyama River near Santa Maria, $2\frac{1}{2}$ miles above Alamo Creek, and the station on the Huasna River near Santa Maria, half a mile above the mouth. (See preceding table.) A gaging station

In the study of seepage losses from streams, numerous miscellaneous measurements and estimates of flow have been made at places along the Cuyama, Sisquoc, and Santa-Maria Rivers and their tributaries. The records of measured discharge at all gaging stations are sum-January 1941.
In the study of seepage losses from streams, numerous m
measurements and estimates of flow have been made at μ
the Cuyama, Sisquoc, and Santa-Maria Rivers and their
The records of measured discharge at all

TABLE 3.—Measured runoff, in acre-feet, at eight gaging stations in the Santa Maria River drainage system in the water years 1904–45

See footnotes at end of table.

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RUNOFF IN THE SANTA MARIA RIVER DRAINAGE SYSTEM GENERAL FEATURES OF THE DRAINAGE BASINS

For purposes of this report, the Santa Maria River drainage system is divided into nine subsidiary drainage areas tributary to the gaging stations, and an ungaged area immediately upstream from Fugler Point (pl. 4). The greater part of this drainage system is included in the drainage basins of the Cuyama and Sisquoc Rivers and their tributaries. Thus, with respect to runoff, the most important of these subareas are the eight above Fugler Point, from all but one of which the runoff has been gaged in recent years. The physical features of these drainage areas differ considerably; they are summarized briefly herewith.

The Cuyama River is the longest stream in the area. Above Ozena it drains a fan-shaped high, mountainous area, about 10 miles long from east to west, which is outside of Santa Barbara County. Below Ozena it flows northwest for about 50 miles across the broad Cuyama Valley, which is bordered on the south by the Sierra Madre and on the north by the Caliente Range. Runoff from these mountains is largely absorbed in the Cuyama Valley, and only in time of flood does the river flow across the full valley reach. Below Gypsum Canyon, however, the river flows southwesterly through a narrow rock gorge for about 20 miles, in a winding course across the axes of both the Sierra Madre and San Rafael Mountains. At the lower end of this reach it is joined by its principal tributaries, Alamo Creek and the Huasna River. These two streams are each between 19 and 20 miles long and, together with the adjacent reach of the Cuyama River, drain the northwestern extension of the San Rafael Mountains and the Sierra Madre. This is a fairly rugged well-watered terrain, which supplies most of the total Cuyama River runoff. Below the mouth of the Huasna, the Cuyama River flows generally south for about 8 miles to Fugler Point, where it leaves the consolidated rock canyon, enters the Santa Maria Valley area, and joins the Sisquoc River.

The Sisquoc River is about 40 miles in total length, the upper 25 miles of which is in the very rugged region between the San Rafael Mountains and the Sierra Madre. (See pl. 4.) It crosses the axis of the San Rafael Mountains about 8 miles above La Brea Creek, and thence flows through lower and less rugged terrane to its confluence with the Cuyama River at Fugler Point. The principal downstream tributary is La Brea Creek, which heads in the Sierra Madre and crosses the northwest extension of the San Rafael Mountains.

The Santa Maria River proper extends from the confluence of the sigvey MinTaard Sisquoc Rivers, at Fugler Point, to the Pacific Ocean and traverses the full length of the Santa Maria plain. That plain is

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primarily an area of water absorption characterized by Iow altitude. gentle land-surface slopes, and relatively light rainfall. Runoff resulting from rainfall on the local drainage area tributary to the Santa Maria River proper forms an insignificant part of the total flow of that stream and thus contributes little to ground-water. replenishment.

On the other hand, runoff from the area drained by the Cuyama, and Sisquoc Rivers supplies at least three-quarters of the recharge to ground water in the valley area. (See tables 5 and 7.) Therefore,. it is one of the basic elements in the hydrologic equation of the valley. In the ensuing paragraphs, only the drainage area above $Fugler$ Point is considered in the discussion of rainfall and runoff.

DISTRIBUTION OF RAINFALL ON THE HEADWATER AREA.

Quantities and intensities of rainfall on the headwater area are for the most part unknown. Prior to 1946 $(p. 58)$ there was no known rain gage in the drainage basin of the Sisquoc River ahove the mouth of La Brea Creek, in the La Brea Creek basin, or in the Alamo Creek basin. Also, in the Huasna River basin rainfall records were not available to provide adequate information for that area. In the Cuyama Valley a long record at Ozena and four short records furnished some information regarding quantities of rainfall in that semiarid region.

In this study, therefore, rainfall distribution among the several stream drainage basins is considered only qualitatively. Suggested distribution is based on the relation of orographic features to storm. paths, type and luxuriance of vegetation, and size and condition of stream channels as related to drainage areas of the respective streams.

The general topographic pattern, as it affects precipitation, is as: follows: The westward-trending San Rafael Mountains form the south watershed of the Sisquoc River basin at altitudes ranging from $4,000$ ⁱ to 6,000 feet. The northwestward extension of the San Rafael Mountains, which is crossed by the Sisquioc River and extends toward the Santa Lucia Mountains near San Luis Obispo, forms the west watershed of the Huasna River basin at altitudes ranging from 1,000 to 3,000 feet. Making an acute angle with the San Rafael Mountains. the northwestward-trending Sierra. Madre separates the Sisquoc River basin from the Cuyama River basin; the altitude of its crestranges from 3,000 to 5,000 feet, with a few peaks higher than 5,000[.] feet. The Cuyama Valley is a long alluvial valley, whose floor ranges. from 1,500 to 3,000 feet above sea level. The Caliente Range north and northeast of that valley is not high enough to have any appreciable effect on precipitation.

Storms along the coast of Santa Barbara County dusurally moves www.NoNewWipPax.com inland from the southwest, west, or northwest. Moist aur moving;

from the south and east acrosses the Santa Ynez Mountains, the San. Rafael Mountains; and the Sierra Madre: On the other hand, moist, air moving from the west and northwest crosses the mountains drained by the Huasna River and Alamo Creek but with a path almost parallel. to the crest of the San Rafael Mountains and the Sierra Madre. Thus, under the first condition of air movement, relatively largeamounts of precipitation may be produced on the Sisquoc drainage basin, whereas under the second condition relatively large amounts of precipitation may be produced on the drainage basins of the Huasna River and Alamo Creek.

Vegetation is heaviest on the north flank of the San Rafael Mountains and is moderately heavy in the Huasna River basin and on the the north flank of the extreme eastern part of the Sierra Madre. Moderate growths of' brush' and grass cover the south flank of the' Sierra Madre, 'but very. little native vegetation is present in the· Cuyama Valley and in the hills north and northeast of that valley. Although factors other than rainfall necessarily affect the type and quantity of vegetation, nevertheless the vegetative pattern closely follows the rainfall-distribution pattern suggested by the relation of the orographic features to storm paths.

. Based on these studies, it appears that average yearly rainfall'is; heaviest on the southern and western parts of the drainage area. becoming progressively lighter toward the north and east. Specifically, some of the heaviest rainfall probably occurs on the north. flank of the San.Rafael Mountains within the south half of the upper Sisquoc River drainage basin. Here, moist air moving in from the south is forced upward over the $4,000$ - to $6,000$ -foot crest of the San Rafael Mountains, with resulting precipitation. It is true that the parallel Santa Ynez Mountains to the south, which rise to altitudes. of 3,000 to 4,000 feet, have already exacted their toll of precipitation as discussed in the companion report on the Santa Ynez River valley. (Upson and Thomasson, 1951). However, the greater altitudeof the San Rafael Mountains may reasonably produce secondary precipitation. Rainfall may be fairly uniform from the Sisquoc River northward to the crest of the Sierra Madre, whence it decreases rapidly down the north flank of that range. Although in places the Sierra Madre is almost as high as the San Rafael Mountains, it apparently has less effect on precipitation.

The Cuyama Valley and lower hills north and east of that valley are very dry, moisture available for precipitation apparently having been intercepted by the mountain ranges to the south. However, considerable rain and snow fall on the high mountains surrounding the extreme eastern part of the Cuyama Valley. For example, Mount Pinos, about 2 miles outside of the basin, is 8,826 feet above.

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sea level and snow collects there in sufficient quantities to provide some runoff. In this area, intense thunderstorms of small extent and of short duration occasionally produce small amounts of flash runoff in the tributary streams. This runoff, however, usually is absorbed in the Cuyama Valley. The Cuyama River is perennial in most years as far downstream as Ozena, but all except large flash flood flows sink before traversing the Cuyama Valley completely.

The northwest part of the Cuyama River drainage system also has considerable rainfall, which probably decreases toward the east. In the absence of any distinct mountain barrier, it may be presumed that average rainfall on the Alamo Oreek drainage basin is less than that on the Huasna River basin, and that the average rainfall on the adjacent small part of the Cuyama drainage basin is in turn less than that on the Alamo Creek drainage basin, but greater than that on the Cuyama Valley proper.

In an effort to relieve the deficiency in basic precipitation data for the mountainous areas of Santa Barbara County, several public agencies are now cooperating in the installation and operation of precipitation stations in those areas: Included among these agencies are Santa BaTbara Oounty, the city of Santa Barbara, Oorps of Engineers of the United States Army, United States Forest Service, United States Weather Bureau, and the United States Geological Survey. During the winter of 1945-46, 6 recording rain gages and 10 storage-type gages were installed. In addition, three snow-rain recording gages were installed in 1946. Of the total number, seven recorders and four storage gages are within the Santa Maria River drainage system. The data obtained from these gages should furnish valuable additional information concerning the principal waterproducing area of the county.

RUNOFF AS A FUNCTION OF RAINFALL

The distribution of rainfall on the whole drainage basin of the Santa Maria Valley is known only in a general way, and its relation to runoff is exceedingly complex, probably even more so than in the Santa Ynez River basin (Upson and Thomasson, 1951). Furthermore, runoff in the Cuyama and Sisquoc Rivers has no direct. relation to runoff in the Santa Ynez River. For example, within the periods of concurrent gaging-station records, storms of sufficient magnitude to produce material runoff have occurred in the Huasna . River and Alamo Creek drainage basins at the same time that light ' precipitation fell on the Santa Ynez River valley. The opposite condition has also been observed.

Because of this and other factors that influence the rainfall-runoff relation, estimates of runoff based on rainfall measured comtside tide with NoNWIPTax.Bom area here under consideration are subject to question.

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RUNOFF FROM THE HEADWATER AREA ESTIMATES OF YEARLY RUNOFF

For the purpose of studying water-supply characteristics, the headwater area was subdivided into main stream and tributary stream drainage basins, as previously indicated. For all these basins, except the one immediately upstream from Fugler Point, some gaging-station. records were available (pl. 4) . Estimates of runoff from the various basins were made in order to supplement the available records.

The water supplies originating in the headwater area include not only the surface flow in the streams but also the underflow, or water percolating through the, channel deposits at' the gaging stations. However, the only gaging station at which underflow was important was the lower station on the Sisquoc River. Estimates of seepage loss above that station were made for years in which a record for that station was available. At the other main stem stations underflow was considered to be negligible. For example, between Gypsum Canyon and Fugler Point the Cuyama River flows in a narrow rock canyon on bedrock, or on a thin veneer of channel deposits. Underflow in that canyon was estimated not to exceed a few hundred acrefeet per year-a quantity so small as to be disregarded in the estimated total yearly runoff. Also, in the Sisquoc channel deposits above the upper gaging station underflow which does not exceed a small fraction of a second-foot is probably all intercepted about 1,000 feet upstream from that gage by a low concrete dam reportedly huilt to bedrock.

As brought out in the discussion of gaging-station records $(p, 50)$; the periods of record on the several streams were so, intermittent that in every year except the two water years $1943-44$ and $1944-45$ one or more of the tributary drainage basins was not gaged. Thus,' in all but these two years computations of total yearly runoff in the two river systems involved estimates of runoff from sizable ungaged areas. Such estimates were based largely on comparison with adjacent gaged drainage areas, modified in some instances by miscellaneous low-water discharge measurements. Runoff was not estimated for . any year during which less than two stream-gaging stations were operated within the area. The estimates therefore span only the 16 years ending September 30, 1930-45, the longest continuous period in which two or more gaging stations were operated.

The ungaged part of the total drainage area was not the same in 'all of the 16 years. For example, during the water-years 1930~33, the ungaged drainage area included the Cuyama River drainage . downstream from the main:-stem gage; except that of the Huasna $\mathbb R$ NoN $\mathbb R$ and the drainage area of the Sisquoc River downstream from

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the upper gage, except the narrow valley floor. During the wateryears *1934-40,* the ungaged area included the same area along the' Ouyama River and all of the Sisquoc drainage basin. During the water years 1941-43, the ungaged area included the same area along the Cuyama River but only the small hilly part of the Sisquoc drainage area downstream from the lower gaging station. Since October 1943, the only ungaged'part of the headwater area was the drainage area downstream from the gaging stations for the Cuyama and Huasna Rivers and Alamo Creek and that downstream from the lower Sisquoc gaging station.

Also, it was found that low-flow characteristics among the several basins varied so widely that runoff relations based on yearly totals for gaged areas were not satisfactory for estimating runoff from ungaged areas. For example, the Huasna River has a high storm runoff but a low summer and autumn flow, whereas the adjacent Alamo Creek has a relatively low storm,runoff but a considerable low-water flow. Thus, the normal yearly runoff from Alamo Creek may be ,about half of the Huasna River runoff, yet in the dry year ending September 3Q, 1934, the total estimated runoff from Alamo Creek, obtained by adding monthly quantities based on miscellaneous measurements, was almost double the measured runoff of the Huasna River. Accordingly, the runoff figures in table 4 were obtained by adding measured monthly runoff from the gaged areas and estimated' monthly runoff from ungaged areas.

TABLE 4.-Measured and estimated yearly runoff, in acre-feet, from the headwater area of the Santa Maria River drainage system in the water years 1930-45

^IIncludes measured runoff of La Brea and Tepusquet Creeks, , Estimated.

, Does not include small diversion above gage.

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The yearly totals of the preceding table are subject to considerable 'error, owing largely to inherent differences between.runoff characteristics of the gaged and ungaged drainage areas; also because on the Sisquoc River poorly controlled estimates of large seepage losses had to be made in some years. For example, during the years 1941-43 . when the Sisquoc River was gaged only 'near Garey, the estimated yearly seepage loss upstream from that station ranged between 10,000 and 15,000 acre-feet, or between 10 and 54 percent of the total yearly discharge at the station. Although the estimates of runoff from ungaged areas, may be considerably in error for individual months or even years, the average yearly runoff for the 16 -year period-91,800, acre-feet-is believed to be reasonably accurate.

The runoff characteristics of the separate gaged drainage areas, together with the basis for comparing runoff of one area with that of another, are given in following paragraphs.'

RUNOFF CHARACTERISTICS OF THE INDIVIDUAL DRAINAGE BASINS

Ouyama River above Alamo Oreek.-Records of measured discharge at the gaging station on the Cuyama River, 3 miles upstream from Alamo Creek, have been obtained since December 1929 and published as "Cuyama River near Santa Maria." No estimates of discharge were necessary because the period of analysis was covered by factual records.

The drainage area above the gaging station is 912 square miles. However, the effective drainage area above the station varied widely from year to year. For example, during normal and dry years little . water left the valley above Gypsum Canyon, and for those years runoff past the station was essentially that from the intervening small mountainous area. On the other hand, during wet years some runoff may have been contributed from the full drainage area above the station. Because of this variation in effective drainage area, the records of runoff at this gaging station did not plot consistently with records at gaging stations on nearby streams. Accordingly, estimates , of runoff from ungaged areas were not based on records of Cuyama River runoff. Those records were used, however, as a guide in limiting the estimates which were based on the records for other nearby streams.

Alamo Creek.-A continuous gaging station has been operated on Alamo Creek, 1.2 miles above its mouth since October 1943, and the records have been published as "Alamo Creek near Santa Maria." Between 1930 and 1943, numerous miscellaneous measurements of discharge were made at the same site in all years except 1932 and 1940. Monthly quantities of runoff during the two years 1944 and 1945 were. allotted against concurrent data for the station on the adjacent,
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Huasna River and a relation between the two drainage basins was obtained, as follows: During months of high flow the runoff from the 'Alamo Creek basin appeared to be about half of that from the Huasna River basin. Runoff was about equal when the monthly total was about 200 acre-feet, but during months of low discharge flow in Alamo Creek was consistently greater than that in the Huasna River. (Because there was no reported surface diversion in either basin, the difference must have been due to natural conditions.

During the water years 1930-43, monthly runoff from the Alamo Creek drainage basin was estimated on the basis of the Huasna River record and the runoff relationship that existed between the two streams during the 2 years of concurrent records. Results so obtained were adjusted for periods of low flow on the basis of available low-water measurements of discharge, but no adjustment was made for months in which floods occurred in the Huasna River. It was found that minor rises in the Huasna River early in the rainy season usually were not accompanied by similar rises in Alamo Creek. On the other hand, fairly heavy rainfall on the Alamo Creek drainage basin was necessary to produce an appreciable rise at any time. Yearly estimates of runoff from the Alamo Creek drainage basin are considered reasonably accurate.

The characteristics of the basin relative to the headwater area as a whole may be summed up as follows: Flood peaks are not great and high flows are of short duration. The stream is clear except for a few days following heavy rainfall. It is reported never to have ceased flowing in the driest years, and in most years flow does not drop below 1 second-foot. In the late summer and autumn of most years flow in the Alamo Creek may equal or exceed the combined flow in the Cuyama and Huasna Rivers. As a tributary of the Cuyama River the Alamo Creek is second in importance only to the Huasna River. Huasna River.—A continuous gaging station has been operated since. December 1929 on the Huasna River, 0.5 mile above its mouth, and the records have been published as "Huasna River near Santa Maria" The period of study was covered by that record. The drainage area above the gage, 119 square miles, is largely mountainous with a small farmed area in the middle part. Storm runoff is flashy and is followed by rapid recession to medium rates of flow. The stream is clear except during floods and is perennial at the gage except in the summer and autumn of consecutive dry years. It is the most important tributary of the Cuyama River.

Records at this site were used as the basis for estimating runoffs from the Alamo Creek drainage basin and from the ungaged area to the south, which includes tributaries of the Sischer River Between the upper gage and Fugler Point and, also, the Cuyama River between. the gage and Fugler Point.

Sisquoc River above upper gage.-Records of discharge of the Sisquoc. River at the upper gaging station (pl. 4) were collected during the. period December 1929 to September 1933 and were published as: "Sisquoc River near Sisquoc." These records did not include diversions that may have been made at a site about 500 feet upstream. Such diversions probably were small but they may account for the periods of no flow during the summer and autumn of those years. Miscellaneous measurements made at the site and on intervening tributaries during 1943 indicated considerable seepage loss from the channel between this site and the gaging station below Tepusquet Creek. The gaging station therefore was reestablished as of October 1943, using the same structures as in the earlier years. The recent. records, however, include diversions and therefore represent the total. runoff above this site.

In estimating runoff during years of no gaging-station record, it was: considered desirable to separate the drainage area above the upper: gage from that below because the rainfall and runoff characteristics. of the two parts were quite different. Quantities of monthly runoff, measured at the upper gaging station, therefore, were plotted against. corresponding quantities of runoff of the Santa Ynez River above; Gibraltar Dam, which were corrected for the operation of Jameson. Lake. That drainage area is immediately adjacent to the Sisquoce on the south. The comparison indicated that the runoff at the Sisquoc. station was about 80 percent of the corresponding runoff above Gibral-: tar Dam. Accordingly, quantities of monthly runoff of the Sisquoci above the upper gage during the period from October 1933 to September 1940 were estimated on the basis of records at Gibraltar Dam. by the use of this relation. Yearly runoff, obtained by adding the. estimated monthly quantities, is considered reasonably accurate.

The part of the Sisquoc River drainage basin above the upper gage. is probably the wettest of all the drainage areas here considered. In some years runoff from the 290 square miles apparently equals that: from the remaining 1,300 square miles in the headwater area. This is not true in all years, however, because of the variation in rainfall. distribution from year to year, but in all years this 290 square miles. of drainage area is a very important contributor to the Santa Maria Valley area.

Sisquoc River above lower gage. A gaging station has been operated; since February 1941 on the Sisquoc River, about 0.5 mile downstream from Tepusquet Creek. The records of runoff, published under the Aeading ratisatioc River near Garey," represent runoff from all the

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drainage area of the Sisquoc River except the small area to the west, between the gage and Fugler Point. However, the records did not include considerable quantities of seepage loss from the channel between the upper and lower gages. Miscellaneous discharge measurements made in 1943 and subsequent gaging-station records on the main stem and tributaries indicated that seepage loss above the gaging station near Garey was about 15 second-feet late in the runoff season. It may have been much greater than this early in the season and also during high flows when considerable areas of the channel were flooded. Because the seepage loss was substantial, and because it varied . from year to year depending on the duration and quantities of total yearly flows, no estimates of prior runoff at this gaging station were prepared. Records at the Garey station were used in the computa-

tions of total inflow to the Santa Maria Valley area only during the water years 1941-45. For earlier years estimates of runoff from the area above the upper station on the Sisquoc River plus runoff from the intervening area between the two stations were considered more. reliable than estimates at the lower station plus estimates of seepage·

loss above it. *La Brea Creek*. ---Records of discharge have been collected since. October 1943 on La Brea Creek, 0.4 mile above the mouth, and published as "La Brea Creek near Sisquoc." In addition, one miscellaneous measurement was made near this site in 1942 and six were $\frac{1}{2}$. made in 1943. The gaging station is on the valley fill about 0.3 mile downstream from the consolidated rock channel and some small seepage loss above the station was not included in the records of October 1943 on La Brea Creek, 0.4 mile above the mouth, and

published as "La Brea Creek near Sisquoc." In addition, one mis-

cellaneous measurement was made near this site in 1942 and six were

made in 1943. The gaging

runoff.
Records of measured runoff from the La Brea Creek drainage basin do not appear directly in table 4 because the concurrent records on the Sisquoc River near Garey include runoff from this basin. The records were used, however, in computing seepage loss above Garey during the 2 years ending September 30, 1945, and in setting up rates. of seepage loss above Garey for use in earlier years. The records were. also combined with records of runoff from the Tepusquet Creek $\frac{35}{28}$ drainage basin to derive runoff relations which were used in estimates. for years predating the period of record for these basins, as discussed Figure 1.1 The hasin is munitained
Surface of seepage loss above Garey for use in earlier years. The records were
also combined with records of runoff from the Tepusquet Creek
drainage basin to derive runoff relations whic

La Brea Oreek drainage basin is uninhabited except for a few stock ranches, which require little water. The basin is mountainous throughout and is characterized by flash runoff, accompanying heavy rainfall, followed by rapid recession to small flows. A small perennial flow is present in most years in the lower reaches of the rock canyon but that flow sinks into the valley fill so that the stream is dry at the sweet and the drain
Copy of document found at its way wip Tax gone mouth during each summer and autumn. During the 2 years of recording

the basin contributed 6 or 7 percent of the total inflow from streams to the valley area.

Tepusquet Creek.-Recofds of 'discharge have been collected since October 1943 on Tepusquet Creek, 1.1 miles above the mouth, and published as "Tepusquet Creek near Sisquoc." Prior to the establishment of the gage miscellaneous measurements were made as follows: one in 1941, one in 1942, and eleven in 1943. The gaging station is in a narrow rock-walled canyon and underflow is negligible. The stream is perennial at the gaging station in most years. Low flows are absorbed within a few hundred feet after reaching the Sisquoc channel. As with the La Brea Creek record, the records of runoff from the Tepusquet Creek drainage basin do not appear in table 4. They were used in conjunction with the La Brea Creek records, as discussed on Tepusquet Creek drainage basin do not appear in table
used in conjunction with the La Brea Creek records, a
pages 64 and 66.
Tepusquet Creek drainage basin is mainly one long ca

Tepusquet Creek drainage basin is mainly one long canyon with fanlike tributaries in the mountainous headwaters. Storm runoff is very small-the basin absorbs all but the heaviest rains. Flow is uniform in winter and holds up until well into the summer, when it slowly recedes to the autumn low. Any diversions above the gage are too small to be detected by diurnal fluctuations in flow at the gaging station. station. The contract of the c

Ungaged area above. Fugler Point.-The preceding discussion has dealt with records and estimates of runoff from the several drainage basins in the headwater area for which records were available in some years. No records of runoff were available for the remainder of the headwater area, which includes downstream segments of both the Cuyama and Sisquoc River drainage basins between the gaging stations and Fugler Point. In order to complete the estimates of total runoff reaching the Santa Maria Valley area from the headwater area, .It was necessary to estimate the runoff from this downstream area for .' all.years.

The characteristics of this part of the headwater area are somewhat different from those of the Alamo Creek drainage basin and the Sisquoc River drainage basin, so that it was not feasible to combine it with either of the others in preparing estimates of runoff. The area consists largely of foothills and mountains of relatively low altitude; having lighter average rainfall than either the Alamo or Sisquoc basins. Because of the staggered periods of record at the various gaging. stations, the downstream ungaged area was not constant. throughout the period of analysis. During the period prior to October 1940 it included the drainage area downstream from the gaging station on the Cuyama River, excluding the Huasna River and Alamo Creek basins. and the drainage area downstream from the upper gage on the Sisquoc- \mathbb{R}^N being that part previously described as being in the valley

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area. Since October 1940 it has included the same area downstream' from the Ouyama gage and the small.hilly part of the area north of the Sisquoc River, and downstream from the lower Sisquoc gage.

. The procedure used for estimating runoff from this area prior to October 1940 was as follows: Monthly runoff during the 2 years 1943-44 and 1944-45 was gaged at the stations on La Brea and Tepusquet Oreeks. Runoff during those 2 years from the ungaged area adjacent to the Ouyama River and from the ungaged area adjacent to the Sisquoc River valley Boor downstream from the upper gage (mostly Foxen Canyon) were each assumed to be about equal to runoff from Tepusquet Creek drainage basin. Thus, runoff during those 2 years from the total area below the Cuyama and upper. Sisquoc gages was estimated as the sum of the runoff of La Brea Creek plus three times the runoff of Tepusquet Creek. Total quantities of monthly runoff so derived were plotted with corresponding quantities of measured runoff of the Huasna River. Although the plotted points scattered considerably, estimated runoff from the area under studyseemed to be about 90 percent of that from the Huasna River drainage· basin. This relation was applied to the records of runoff of the Huasna River prior to October 1940, to estimate quantities of runoff from the ungaged area above Fugler Point for the equivalent period. The runoff computed by this procedure was found to be unreasonably high for several months as compared to estimated runoff in the adjacent drainage basin of the upper Sisquoc River, which had been computed from records for the Santa Ynez River. The runoff was adjusted arbitrarily for those months so as not to exceed 50 percent of the estimated runoff from the upper Sisquoc drainage basin. This adjustment was made on the basis of the relation between runoff of the two areas during the last 2 years when gaging station records were available, probable rainfall distribution on the two areas, and the fact that the estimates of runoff for the Sisquoc were based on more and better. factual data.

For the water years 1941-45, the ungaged balance of the headwater area included the area downstream from the Ouyama gage and small hilly area downstream from the lower Sisquoc gage and north of the river. For the period October 1940 to September 1943, runoff from the ungaged balance was arbitrarily estimated on the basis of unit runoff from adjacent areas. For the period October 1943 to September 1945, runoff from this area was assumed to have been: about the same as the measured runoff from the Tepusquet Oreek basin in the period October 1940 to September 1943, runoff
from the ungaged balance was arbitrarily estimated on the basis of
mit runoff from adjacent areas. For the period October 1943 to
September 1945, runoff from this

Estimates of runoff in all years from the ungaged area immediately upstream from Fugler Point are considered good document at ω at ω and ω at ω as quantities represent only about 10 percent of the total inflow to the

Santa Maria Valley area during the period prior to October 1940 and only 2 or 3 percent of the total during the later years. The inaccuracies in the estimates, therefore, do not introduce very material errors into the estimates of total infloW-.

SEEPAGE LOSSES FROM STREAMS

At the start of this investigation it was known that water was lost by seepage from stream channels in the Santa Maria Valley area. The evaluation of such losses was an integral part of this study of the water resources of the valley. The difference between the total inflow from the headwater area, which has been summarized in the foregoing paragraphs, and the total surface-water outflow to the sea was considered to be approximately equal to the total seepage losses within the valley. The runoff from the valley area proper was quite small and no allowance was made for it in the seepage studies.

Numerous miscellaneous measurements and estimates of stream discharge indicated that principal losses occurred in the reach between the upper gage on the Sisquoc River and the inland edge of the artesian area on the Santa Maria River (pI. 5). Minor losses were noted downstream from the Cuyama River gage to its confluence with the Sisquoc River at Fugler Point. These reaches of stream channel are within the recharge area as defined in this report (p. 73). In the western part of the valley area the Santa Maria River is separated from the main ground-water body by confining beds, and very little permanent seepage loss occurs. Stream flow reaching the area of confined ground water is largely wasted to the ocean.

Surface-water outflow was largely measured at a gaging station installed on the Santa Maria River at Guadalupe in January 1941 (table 3), well within the artesian area (pl. 5). Considerable difficulty has been experienced in the operation of this station. During periods when the river was flowing, the stream has continually shifted back and forth across the wide sand channel so that gage heights have been uncertain, when recorded at all. In some recorded years the flow at the station was so small it never registered on the gage. Discharge records during those periods when the stream was away from the gage were computed largely on the basis of the composite inBow hydrograph adjusted to discharge measurements made at Guadalupe.

Even though the discharge records at Guadalupe were rated no better than "poor," nevertheless they furnished considerable valuable information regarding rates and quantities of seepage loss above that station. For example, in the year ending September 30, 1942, the measured and estimated total inflow to the valley was 52,600 acrefeet, but the outflow in that year as measured at Guadalupe was $^{\rm 3000}$ acre-feet; or the loss from stream channels above Guada- $\lceil \text{lupe was about 51,500 acre-feet.} \rceil$ Similarly, during several months the

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total inflow was more than 15,000 acre-feet, whereas the outflowwasfrom zero to a few hundred acre-feet. Thus, sizable errors in the Guadalupe record would have made no material change in the estimates of total seepage losses for those periods.

A study of several storm periods indicated that seepage loss from streams exceeded 1,000 acre-feet per day for moderate flows and that it probably exceeded 2,000 acre-feet per day during major floods when large areas of channel were flooded. In view of the large and variable rates of seepage loss above Guadalupe, it was concluded that for years prior to the period of record reliable estimates of stream flow at that station could not be based on flow at the upstream gaging stations; rather, it appeared that direct estimates of seepage loss during those years would be more accurate.

Accordingly, monthly estimated quantities of seepage loss during the water-years 1941-45 were plotted against corresponding estimated quantities of total inflow to the valley floor from the headwater area. The following significant relations were established from this study: For all months with less than 10,000 acre-feet of total inflow there was no outflow at Guadalupe, indicating that (neglecting evapotranspira-, tion; see p. 71) all inflow sank into the ground. For months in which flow was uniform, with no major floods, the amount of inflow might be as much as 20,000 acre-feet without any outflow at Guadalupe. A good example was March 1944, during which month a moderate flow was sustained and no large flood occurred. Estimated total inflow for the manth was 34,200 acre-feet, but, because it was well distributed with respect to time, only 5,610 acre-feet passed Guadalupe, or 28,600^t acre-feet seeped out of the channel. Rises occurring late in some months caused scattering of the plotted points, owing to channelstorage, and because flash floods may have exceeded the capacity of the channel to absorb water. For example, in 1943, when all streams were low until January 21, the sudden flood beginning that day was great enough to bring the estimated total inflow for the month up to 55,600 acre-feet. Probably more than, 90 percent of the total inflow occurred during the latter third of the month, and the high peak flows. from that storm greatly exceeded the maximum absorption capacity of the channel. Consequently the excess water was wasted to the ocean, and the estimated total seepage loss for this month was only, 20.100 acre-feet.

Just as the monthly quantities of seepage loss depend on the distribution of inflow with respect to time as well as on its total monthly amount, so also the yearly quantities of seepage loss depend on the monthly distribution of inflow as well as on the total yearly amount of inflow. During years in which rainfall was well distributed and copy of document found at weall we will be the considerably greater weally at all the major floods occurred, seepage losses were considerably greater

than during years in which the total inflow was about the same but in which mast of the inflow was concentrated in one or two major floods.

. On the basis .of these principles, quantities of monthly seepage loss for the years ending September 30, 1930-40, were estimated as follows: For all months in which total inflow was less than 10,000 acre-feet, the entire inflow was considered to have seeped from the channel. For months in which total inflow exceeded 10,000 acre-feet, the daily records of flow at available gaging stations in the area, and also in adjacent stream basins, were studied to determine the presence or absence of floods and the distribution of flow with resepct to time. Seepage losses for those months were adjusted for excessive floods and for floods occurring near the end of the months. Estimated $\text{year}[\mathbf{v}]$ seepage loss was then'obtained by adding the monthly estimates.

Table 5 presents estimated yearly inflow to, outflow from, and seepage losses within the Santa Maria Valley area in the 16 years ending September 30, 1945. Quantities of estimated yearly inflow were obtained from table 4. Outflow was measured beginning in 1940-41 (table 3), The quantities of estimated yearly outilow during the years prior to 1940-41, as given in table 5, represent the residual difference between estimated inflow and estimated seepage loss during the respective years. Yearly seepage losses during the years prior to

lew Odtflow measured at Guadalup

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1940-41 were estimated directly, as described in the preceding paragraphs. For the years beginning 1940-41 seepage loss is the residual difference between estimated inflow and measured outflow.

Figure 2 presents in graphic form the estimated average monthly quantities of inflow, seepage loss, and surface-water outflow for the Santa Maria Valley area during the 16-year period analyzed in this report. The two graphs on that plate are based on the same data. The monthly hydrograph shows the distribution of quantities with respect to time but the mass diagram, or cumulative monthly hydrograph, shows more clearly the division of inflow into seepage loss and surface-water outflow.

Estimated seepage losses during months of low and moderate flow are as accurate as the estimated quantities of total inflow. Estimated losses during floods are subject to question but are as accurate as available data permit. Future records of discharge at the present gaging stations will furnish data which will either confirm these estimates or establish a factual basis for their revision.

The over-all difference between surface-water inflow and outflow was classified as seepage loss in table 5. Actually some water evaporated from the water surface in the streams and from the channel sands, and transpired through riparian vegetation, and so did not reach the main water body of the Santa Maria Valley area. Such losses by evapotranspiration are believed not to have exceeded a few hundred acre-feet a year, however, and so were not deducted from the over-all difference.

Under natural conditions of stream regimen that prevailed during the period of current analysis, most of the runoff and seepage loss toolr place during the winter and early spring months, when evapotranspiration losses were at a minimum. However, the disturbance of the natural regimen, in which flood waters might be detained in surface reservoirs and later released during the summer, would result in disproportionately large losses by evaporation from the reservoir surfaces and from stream channels. For example, in the headwaters of the Santa Ynez River, about 50 miles southeast of the Santa Maria. Valley, the 14-year average evaporation at two stations, as measured by class A land pans, was 1.10 and 1.28 inches, respectively, during the month of January, and 8.08 and 9.80 inches, respectively, during the month of July (Upson and Thomasson, in preparation). Furthermore, the conservation of flood waters through the use of storage reservoirs would probably produce large additional losses by transpiration. Water-loving plants around the edges of the reservoirs would take their toll and, also, many acres of riparian vegetation could be expected Conv of document found at solid More SERFERX Bonand flourish along the stream channels, which now are

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mostly barren, if furnished an adequate supply of water during the growing season.

Although evapotranspiration losses might be increased manyfold $b\gamma$ the regulation of the stream regimen, such losses could be minimized by releasing the stored flood waters as rapidly. as the stream channels could be made to absorb ·them. The increased evapotrans- .piration losses resulting from reservoir storage undoubtedly would be far more than offset by the reduction in peak flows and hence the salvage of water that otherwise would waste to the ocean. The stream system appears to be well suited to the development of dualpurpose reservoirs for the control of floods and the conservation of water supplies.

GROUND-WATER RESOURCES

This evaluation of the ground-water resources of the Santa Maria Valley area is developed through successive treatment of the occurrence of ground water essentially in a single main water body, its magnitude and its containing formations, and conditions which cause its partial confinement; the source and movement of water through the deposits, with a critical analysis of the controlling factors; the nature and quantity of recharge; the nature and quantity of discharge; water-level fluctuations and their relation to net changes in groundwater storage; estimates of perennial yield of the basin; and finally the general chemical quality of water and possibilities of sea-water encroachment. The quantitative hydrologic studies are limited to the period 1929-45 by the availability of records on water levels, rainfall, runoff, and pumpage.

OCCURRENCE. OF GROUND WATER MAIN WATER BODY

The main water body of the Santa Maria Valley area extends continuously from the head of the Sisquoc plain on the east to the Pacific Ocean on the west and is contained within the unconsolidated deposits that fill the major syncline, described on page 42. Minor arms extend up the tributary tongues of alluvial material, principally those along the Cuyama and Sisquoc Rivers. The containing formations include the alluvium, the Orcutt and Paso Robles formations, and the Careaga sand; also, locally, the terrace and channel deposits. $(pl. 1)$. The bottom of the water body is considered to be at the base of the Careaga sand. In the deeper parts of the basin the water may be of poor chemical quality.

This main water body is as much as 8% miles wide and underlies. an area of about 110,000 acres. Its maximum thickness is about 1,300 feet beneath the Sisquoc plain near Sisquoc and 2,800 feet was to of the Orcument of the Copy of

beneath the Orcutt upland near Orcutt; however, the average thickness is roughly 1,000 feet. Thus, the total volume of saturated deposits is roughly $100,000,000$ acre-feet. Unfortunately, only a very small :part of the total volume of water in the containing deposits can be withdrawn for use without exceeding the perennial yielding (p. 123).

Most wells. penetrate only from 200 to 400 feet into the main water body; they disclose no marked differences in head of the water within that range of penetration. With respect to the land surface, in general the head in wells ranges from about 500 feet below in the southeast partof the Orcutt upland to about 10 feet above near the coast. Any minor differences of head which may exist between the several formations tapped probably are largely equalized within the casings of wells which tap more than one formation. In the few deep wells perforated only in the Paso Robles formation, the head is a few feet higher than in shallower wells tapping only the overlying formations. This slight increase of head with depth probably is due to local confinement of water beneath clay lenses in the Paso Robles. Plate 5 shows contours on the water table or pressure surface of the main body.

Beneath the eastern and larger part of the area about 80;000 acres. of the main water body is unconfined; however, beneath the western part of the Santa Maria plain about 30,000 acres is confined beneath the upper member of the alluvium. In turn, the area of confined water has two parts-an eastern part where the head of water is below the land surface, and a western part where the head is above the land sur- 'face and where there are flowing wells. The extent of the area of flowing wells has Yaried considerably during the past 27 years, as is shown on plate 5. .

The eastern boundary of confined water is somewhat irregular and intangible, but in general, it is roughly along the line between Rs. 34 and 35 W. (See pIs. '1 and 5.) This position is deduced chiefly from physical and lithologic features of the upper member of the alluvium, from differences in the fluctuation of water levels in wells. and from the reported areas of ground-water discharge as of 1918.

The area of unconfined water is one of potential recharge, and is called the intake area because there water is able to infiltrate from the land surface down to the water table of the main water body. On the . other hand, in the area of confined water, there is essentially no infiltration from the land surface because of the low permeability of the confining beds.

MlNOR WATER BODIES

In the Santa Maria Valley area there are three known minor water bodies, as follows:

Newthen A thin and possibly discontinuous body beneath the central part of the Orcutt upland, contained in dune sand. It is perched above the

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main water body on fine-grained deposits or old soils of the Orcutt formation, and it supplies water in small quantities to a few domestic wells, Recharge is wholly by infiltration of rain and water not withdrawn or retained in storage eventually reaches the main water body below.

2, A relatively thin body beneath the Nipomo upland; contained in the terrace deposits and upheld by consolidated rocks; Wells tapping' this body yield water in quantities sufficient only for domestic and stock' needs, Recharge is principally from rain but partly from minor streams, South of the drainage divide water that is not extracted moves southwest through the deposits and eventually reaches the main water body. (See pl. 2, sec. $D-D'$.)

3. A shallow body in the uppermost part of the alluvium and in the channel deposits in the area of main-body confinement; and extending into the dune sand at the west end of the Santa Maria plain. Recharge is chiefly by seepage from streams, and infiltration of rain and irrigation water. Discharge, which takes place by drainage westward. toward the ocean, sustains the perennial dry-season flow in the lower reaches of the Santa Maria River and Oso Flaco Creek. No wells tap this body,

SOURCE AND MOVEMENT OF GROUND WATER GENERAL FEATURES SHOWN BY WATER-LEVEL CONTOURS

The sources of ground-water recharge are indicated by the direction of movement of water in the main water body, Water moves awayfrom areas of replenishment toward points of discharge, Specifically, provided impermeable barriers do not exist,' movement is indicated by differences of head between any two points because water always moves from a point of high head to' a point of low head, Oontour lines drawn on the surface of a water body connect points of equal head.

Plate 5 shows by contours the head of water throughout the main water body, based on measurements of "static" (nonpumping) levels in wells made in February to May of 1936 and 1942. Those for 1942 are based on measurements made and compiled by the Geological . Survey, and those for 1936 were supplied by several agencies in the Santa Maria Valley. Altitudes of wells were determined by spirit leveling or aneroid barometer, or weie interpolated from topographic maps. Within the area of confined water the contours are drawn on the pressure surface of the main water body, and elsewhere on the water table.

The contours for 1942 show the head of water during a period of relatively high water levels, and those for 1936 show the head during the lowest period of record. The map also shows the approximate eastern \lim it of the area of flowing wells in 1918, 1936, and 1942, and the reported areas of ground-water overflow in 1918.

With respect to source, both sets of contours show that within the intake area water moves generally westward away from the Sisquoc and Santa Maria Rivers: water moves northward away from the Casmalia and Solomon Hills, and southward from the western part of the Nipomo upland; and water moves down the lower course of the Cuvama River. Similar movement down the canyon of the Sisquoc is indicated by maps not here reproduced, In other words, the contours show that substantial recharge to the main water body is accomplished by seepage from streams, infiltration of rain on the bordering hills) and underflow in the alluvium and channel deposits. along the rivers. Considerable recharge also is accomplished by infiltration of rain on the intake area, but this is so thoroughly dispersed that it is not shown by the contours.

Although the shape of the contours on plate 5 is influenced chiefly by recharge, it is modified also by conditions within the main water body, such as changes in, permeability of the containing deposits and changes in cross-sectional area of the deposits, and by fault barriers. The variations in the movement of water through the area as caused by these structural and lithologic features, and hence the changes in the configuration of the contours, are discussed separately as follows:

MOVEMENT IN THE SISQUOC VALLEY

The contour map (pl. 5) shows that beneath the Sisquoc plain water is moving with a fairly uniform hydraulic gradient in a westerly direction. The direction is established by the natural westward drainage and the withdrawals for irrigation farther west. Along the north and south sides of the plain few data are available concerning the movement, but it is presumed to be towards the plain.

PERCOLATION FROM THE SISQUOC VALLEY TO THE SANTA MARIA VALLEY

The movement of water from the Sisquoc valley to the Santa Maria Valley takes place through the Careaga sand, the Paso Robles formation, and possibly the terrace deposits on the south side of Fugler . Point, and principally through the alluvium on the north side. At Fugler point the main water body is split longitudinally by the out cropping tar-impregnated Careaga sand, which forms an impermeable "island" at the north end of the Point, (See geologic sec. $B-B'$, pl. 2.)

· . The water-level contour map shows that south of Fugler Point, a. rapid steepening of gradient occurs immediately west of Garey from about 25 feet per mile to about 100 feet per mile.. There are several possible explanations for· this featme, and among the most lil{ely are. Copy of document found at www.NoNewWipTax.com

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the following: a large decrease in the permeability of the saturated deposits as water moves from the highly permeable alluvium to the less permeable older formations-thus; in order to transmit the same quantity of water through the less permeable deposits a steeper hydraulic gradient would be required; the inferred westward plunge of the base of the permeable beds west of Garey which might cause a . steepening of hydraulic gradient; and the presence of the postulated, Fugler Point fault (p. 43) which might retard the movement 'by a reduction in effective cross-sectional area through displacement of beds, through cementation, or through impregnation by tar seepage. ' Until more data are available on the existence of the fault, the first two explanations together are believed to be the most reasonable.

In the alluvium on the north side of Fugler Point, on the other hand, the water table has approximately the same gradient as established in the Sisquoc valley but steepens rapidly below. Thus, there exists relatively free hydraulic continuity between the Sisquoc and Santa Maria valleys through the alluvium.

The quantity of water moving as underflow from the Sisquoc valley. to the Santa Maria Valley through the alluvium is of particular interest, especially in view of possible future water-spreading operations in the channels, and the subsequent transmission of water stored in the deposits of the Sisquoc valley. The amounts of underflow for the two years 1936 and 1944 are used to show the extremes of maximum and minimum values, respectively, and are determined by the use of Darcy's law, which may be expressed by the formula

Q=PJA,

in which Q is the quantity of water in gallons per day, P is the permeability coefficient in gallons per day per square foot, I is the hydraulic gradient in feet per mile, and A is the cross-sectional area in square feet (Wenzel, 1942, pp. 3-4). The permeability coefficient used is $3,500$ gallons a day per square foot, obtained from the test on well $10/33-21R1$ (p. 38). Obviously, at best, this value is only an estimate because it was determined in an area 2 miles downstream, and further it may not apply strictly to both years when the deposits were saturated to different depths.

In 1936, the hydraulic gradient was about 15 feet per mile. (See pls. 5 and 6.) The width of the saturated deposits was about $3,000$ feet and the thickness about 60 feet, giving a total saturated crosssectional area of about 180,000 square feet. The quantity of water moving through these deposits in 1936 is computed to have been about 1,800,000 gallons a day-about 2.8 second-feet, or 2,000 acrefeet a year.

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Similarly, in 1944 the hydraulic gradient was about 20 feet per mile. The width was the same, 3,000 feet, and the saturated thickness was about 100 feet, giving a total saturated cross-sectional area of 300,000 square feet. The quantity of water moving through the deposits then was about $4,000,000$ gallons a day, about 6.2 second-feet, or 4.500 acre-feet a year. Thus the natural limits of underflow have ranged from about 2,000 to 4,500 acre-feet a year. .

MOVEMENT IN THE SANTA MARIA VALLEY.

The water-level contour maps (pl. 5) show three main features in regard to the movement of ground water in the Santa Maria Valley, as follows: a striking longitudinal break or flattening of hydraulic gradient near the central part of the valley; a wide lateral shifting of the trough, or low, down the middle of the valley between 1936 and 1942; and a seaward gradient to and at the coast.

The longitudinal break in hydraulic gradient near the central part of the valley, which in,1942. was from about 40 feet per mile on the east to less than 10 feet per mile on the west, is evident as far back as 1907 when there was little pumping in the area. This is clearly shown by the profiles of water levels for selected years (pl. 6). Consequently, the break is a natural phenomenon and not the result of pumping. Furthermore, the break is not the result of displacement of beds along the Santa Maria fault, because the fault does not cut the upper part of the main water body contained in the Orcutt formation nor in the highly permeable alluvium; because the displace~' merit of' the older unconsolidated' deposits is small and does not materially alter the cross-sectional area; and further because the change in gradient is just the reverse of that which would be produced by a fault barrier. '

The flattening is believed to be due primarily to a line of hydraulic balance established at the intersection of two independently controlled gradients: the western gentle gradient, which is controlled largely by the rate of discharge at the coast, and the eastern steep gradient, which is determined largely by the rate of recharge from the Santa Maria River and from underflow out of the· Sisquoc valley; in conjunction .. with considerable widening and thickening of the water-bearing deposits from the Sisquoc valley westward to the central part of the Santa Maria Valley. (See geologic sec. $B-B'$, $C-C'$, and $D-D'$, pl. 2.) The water-level profiles (pl. 6) show that the line of balance has shifted only slightly eastward or westward since 1907, depending upon the controlling altitudes of the water surface at either end of the valley. From the edge of the area of confined water westward the gradient steepens slightly, probably owing to a decrease in cross-Copy of document found at ware NoNewWipTax area, of the water-bearing deposits.

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The second feature of water movement in the Santa Maria Valley is the lateral shifting of the trough on the surface of the water body shown by two sets of contours on plate 5. In 1936 the trough ex tended roughly up the central part of the Santa Maria ·plain, crossed beneath the Orcutt upland . southeast of Santa Maria, and probably entered the Sisquoc valley near Garey. In 1942 the trough had moved southward a maximum distance of about 5 miles and extended along; the south side of the, Orcutt upland, entering the Sisquoc valley near Garey. Thus at present the trough lies about 3 miles south from the Santa Maria plain in an area where only about 5 petcent of the with- . drawals occur, and hence is not simply a pumping depression. .

Its position is probably determined primarily by the relation between recharge from the Santa Maria River and discharge by pumping from beneath the Santa Maria plain. In the long series of d_{TV} vears ending in 1936, there was relatively small recharge from the river. Consequently, pumpage exceeded recharge and the trough shifted northward from beneath the Orcutt uplimd toward the center of pumping. On the other hand, in 1942, following a period of wet. years, recharge from the river exceeded withdrawals on the plain, and· the excess water moved southward beneath the Orcutt upland, causing the trough to shift in that direction. Thus, the source and movement of water in the heavily pumped area may vary over a period of years. During wet years recharge from the Santa Maria River supplies more water than is pumped, but during dry years water supplied by the river is inadequate and the water beneath the Orcutt upland, which is supplied by infiltration of rain, is more heavily drawn upon.

The third feature, the seaward hydraulic gradient of the main water body to and at the coast, is extremely important because it means that water is moving toward and is being discharged into the Pacific Ocean at some point off the coast, and it is thereby preventing the landward encroachment of sea water. The water-level profiles (pl. 6) show the hydraulic gradients for the various years projected to the' coast line. If extended seaward, they indicate that the point of discharge is somewhere between 2 and 4 miles off shore. The profiles, also indicate that there has always been escape at the coast.

RECHARGE TO THE MAIN WATER BODY

In some areas, such as those in the Midwestern States, recharge to. water-bearing formations from rain and streams may take place in remote districts hundreds of miles from the points of withdrawal. In such areas the evaluation of quantities of annual or long-term recharge involves chiefly computations of the amount of ground water transmitted into the areas through the aquifers. In the Santa Maria. Valley area, on the other hand, practically all the recharge takes place, $\frac{1}{2}$ where $\frac{1}{2}$

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within the boundaries of the area shown on the geologic map (pl. 1). Therefore, in order to determine total recharge it is necessary to estimate recharge to the main water body by appraising seepage loss from streams, infiltration of rain and underflow along principal streams.

SEEPAGE FROM STREAMS

As indicated on pages 67 and 73, stream losses take place in the lower course of the Cuyama River, in the Sisquoc River below the upper gage, and in the Santa Maria River downstream to the area of confined water. Within this area there are no extensive impermeable beds, and water seeping from the streams is able to reach the main 'water body. Large losses are possible because of the relatively high permeability of the channel deposits, which ranges from 266 to over $1,000$ gallons a day per square foot (table 2), and the large areas from which the losses can take place—about $2,700$ acres in the Sisquoc valley and about 6,300 acres in the Santa Maria Valley, or a total of about 9,000 acres. However, the entire acreage is covered only during infrequent major floods and then for relatively short periods of time. Seepage at most times is from much smaller areas.

Throughout most of the reach in which seepage losses from streams occur, measurements of water levels in wells adjacent to the channels show that the water table lies at considerable depth below the river ·channels .. In the Sisquoc valley the depth has ranged from a minimum of less than a foot at the upper and lower ends to a maximum of 90 feet near Sisquoc. Similarly, in the Santa Maria Valley the depth has ranged from less than a foot at Fugler Point to a maximum of 130 feet north of Santa Maria (pl. 6). Therefore, except near Fugler Point and probably in part of the Sisquoc valley, river water has not. been in hydraulic continuity with the main water body. Water from . the river, then, seeps vertically downward through the permeable channel deposits and through the greater part of the upper member of the alluvium before reaching the main water body as recharge.

The methods used to estimate seepage losses have been presented in the section on surface-water resources, and yearly estimates therein derived for the 16-year period 1930-45 are shown in table 5. The magnitUde of the losses. involved with respect to time are discussed on pages 67 to 79. Because there are but very few water-loving plants along the channel courses, and because evaporation losses during the winter months are at a minimum, for all practical purposes the total yearly estimated seepage losses reach the main water body as recharge in the manner described above. Thus, estimated yearly recharge by . seepage from streanis has ranged from 4,800 acre-feet in 1930-31 to 150,000 acre-feet in 1940-41, and has averaged 56,400 acre-feet for 'the period 1930-45. Recharge from· this source constitutes about 80

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percent of the total recharge to the main water body. Even so, the average surface-water outflow, or the water forever. lost from the basin, has averaged about $35,000$ acre-feet a year during the past 16 years. Therefore, it is obvious that any future plan devised to utilize fully the surface-w'ater resources should consider the advantages of salvaging this wasted water insofar as possible and of spreading it on the appropriate portions of the Sisquoc, Cuyama, and Santa Maria River channels.

In the area of confined water, seepage loss cannot penetrate below the contact between the channel deposits and the relatively impermeable upper member of the alluvium. Water lost here is stored temporarily in the surficial sediments adjacent to the river during high flows, and returns to the stream channels when the floods subside.

INFILTRATION OF RAiN AREAS OF INFILTRATION

The area of rain infiltration encompasses the greater part of the area shown on plate I, and hence is nearly wholly outside the headwater area for which estimates of runoff have been made. It is, estimated to be about 140,000 acres in extent, and receives relatively little rainfall and essentially no runoff from minor tributaries except during infrequent heavy storms. Because the quantity of infiltration is governed principally by the character of the underlying deposits and the type of vegetative cover, the total area of infiltration is divided into a primary area, which is coextensive with the intake area and which includes about 80,000 acres whose cover consists of grass and irrigated lands; and a secondary area, which includes about. . 60,000 acres characterized by thick grow.ths of brush, scrub oak, and some grass, and underlain principally by consolidated rocks.

METHODS USED TO ESTIMATE INFILTRATION

Precise field· determinations of that part of the total rainfall that infiltrates below the root zone and reaches the main water body were beyond the scope of this investigation, and to be of value they would have to be made under a variety of conditions over a series of years. Therefore, the estimates of infiltration are based primarily on field studies made in Ventura Oounty, principally by Blaney: . (Blaney, 1933; Blaney and Sopp, 1929). Although conditions are not exactly the same in the Santa Maria Valley as in Ventura County, it is. believed that they are sufficiently similar for the estimates to be valid.

The primary area is divided into two subareas according to type of land cover: $60,000$ acres of grass land and $20,000$ acres of irrigated land. Infiltration of rain on these lands was determined by plotting a curve of infiltration against rainfall for each type of cover, definition with NoNewWipTax.tom

from data of Blaney, who found that in general there is no infiltration when yearly rainfall is less than 15 inches on grass land or less than 12 inches on irrigated land. From the curve for any given yearly rainfall the infiltration of rain in inches can be estimated. , In this way .the infiltration of rain on each type of land has been estimated for the years 1930-45.

The secondary area is underlain locally by relatively thin terrace deposits and nearly everywhere has a soil mantle that ranges in thickness from 1 foot to 4 feet and which. supports relatively thick growths of brush, scrub oak, and some grass. Because it is underlain mostly by consolidated rock, the principles governing the infiltration of rain are believed not to be the same as in the primary area; rather, the infiltrate must move laterally toward the basin through the soil zone and through fractures near the surface of the consolidated rock, and by so doing it is subject to use by vegetation. Accordingly, the amount reaching the basin is believed to be quite small. The waterlevel contour map (pl. 5) shows water moving northward from the Casmalia and Solomon Hills, indicating that some infiltrate is reaching the primary area by lateral movement. .

Blaney has indicated that in general when yearly rainfall on brush land is less than 18 inches no deep infiltration occurs. It is thought that in the secondary area about 10 percent of the rainfall in excess of 18 inches might be a reasonable estimate for recharge. Accordingly, infiltration each year from this area is taken as 10 percent of the excess over 18 inches when the yearly rainfall is more than 18 inches, .and zero when it is less. Thus, during the period 1930-45, recharge from the area is estimated to have occurred only in 4 years -1935 , 1937, 1938, and 1941 (table 6). For these years; the infiltration is estimated to have ranged from a minimum of about 800 acre-feet in 1935 to a maximum of about $6,400$ acre-feet in 1941, which is only a small part of the totals for those 2 years. Also, the estimated . infiltration includes any recharge that might be supplied by local runoff or by percolation through fractures in the consolidated rocks (p.27). .

It should be pointed out that with years having the same total rainfall there is likely to be a difference in the' amount infiltrating to storage, due to variations in storm intensities, in soil moisture at the beginning of and during the rainy season, and in other related characteristics. Thus, rigid use of the method is subject to some error in anyone year. However, over a series of years these errors would tend to balance each other, and so are used without adjustment.

ESTIMATES OF INFILTRATION

Inflowed and the main water body in any one year, then, is the . sum of the values for each of the three types of land cover obtained

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by the method outlined. The yearly rainfall at Santa Maria is used for all three areas because it is believed to represent about the average for them. At Santa Maria the long-term average rainfall is 14.40 inches ,(table 1). This is less than that required for infiltration on grass and brush lands, but somewhat greater than' that required for irrigated land. Table 6 shows the total recharge thus derived for the years ending September 30, 1930-45. average for them. At Santa Maria the long-term average ranntal is
14.40 inches (table 1). This is less than that required for infiltration
on grass and brush lands, but somewhat greater than that required
for irrigated lan

^lFrom table 1.

i I' !

> The table suggests that there was no infiltration of rain during years. of low rainfall, and that infiltration was about 80,000 acre-feet in 1941, the wettest year of record. The estimated average yearly infiltration' was nearly $14,000$ acre-feet and suggests that about 2 inches per year or about 13 percent of the average rainfall for the 16-year period, infiltrated to storage. However, it is apparent that the average is raised appreciably by the large infiltration that occurred during 1941. For the 50-year period 1886-1945, by the procedure outlined above, it was estimated that the average yearly infiltration was 10,000 acrefeet or 1.5 inches—about 10 percent of the average rainfall for that period. This is about 25 percent less than the average yearly infiltration during the 16-year period 1930-45.

UNDERFLOW ALONG PRINCIPAL STREAMS

The continuous unseen flow of ground water into the main water body, principally through the alluvium at the mouths of the Ouyama and Sisquoc Rivers and major tributaries, is designated as recharge underflow. Essentially all the underflow at the mouths of these rivers is measured as surface flow at stream-gaging stations a considerable distance upstream, where the deposits are thin or missing entirely, and where the underflow is estimated to be only a few hundred acre-feet a year (p. 59). This rough estimate of underflow is well within the limits complex NoNewWipHax.com

of error involved in the estimates of yearly runoff and seepage loss. Consequently, for all practical purposes the recharge by underflow is accounted for in the measured and estimated seepage losses from streams (table 5), and hence is not separately estimated.

ESTIMATE OF TOTAL RECHARGE

The total quantity of recharge to the main water body is the sum of the seepage loss from streams and the infiltration of rain (tables 5) and 6). Table 7 shows the estimates of total yearly recharge for the years ending September 30, 1930-45. .

TABLE 7.-Estimates of total yearly recharge to the main water body in the water *years 1930-45 .*

The table shows that estimated total yearly recharge has ranged from about 4,800 acre-feet in 1931 to 230,000 acre-feet in 1941, and has averaged about 70,000 acre-feet. Thus, anyone year's recharge may be as much as 330 percent of the 16-year average, as in 1941, or as little as 7 percent, as in 1931. Obviously, the large increment in 1941 has raised the average considerably. With respect to long-term average recharge to the main water body based on comparative rainfall, it is about 93 percent of that for the period 1930-45 (p. 128), or is estimated to be about 65,000 acre-feet a year.

For short periods, too, the recharge is roughly proportional to rainfall. For example, the average yearly recharge during the 7-year period 1930-36. was about 34,000 acre-feet, in contrast to an average during the 9-year period 1937-45 of about 98,000 acre-feet. The wide range in average recharge between these two periods can be traced directly to rainfall. For the two periods, the average yearly rainfall was 12.41 inches and 17.77 inches, respectively. Thus, there exists a general relationship between rainfall and total recharge, but because of the relatively wide variation in distribution and intensity of rainfall $($ p. 58) no attempt is made to construct total yearly recharge from rainfall along combe relationship is used, however, in the estimation of long-term average recharge above and of perennial yield (p. 128).

. 'GROWD-WATER RESOURCES .' **I,** ⁸⁵

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DISCHARGE FROM THE MAIN WATER BODY

Discharge of ground water from the main water body occurs in two . ways: by natural means, and by withdrawals from wells, which' include the discharge from uncapped flowing wells, Essentially all discharge of ground water occurred by natural processes prior to the introduction of large-capacity pumps near the turn of the century. Since then pumpage has increased steadily, until in recent years it has constituted about 85 percent of the total discharge. During the past 20 years natural discharge has been only in the form of ground-water outflow to the sea, but in earlier years ground water overflowed at the eastern edge of the area of confined water, and considerable discharge took place. above ground.

PUMPAGE

HISTORY AND DEVELOPMENT

The first recorded well in the area was a dug domestic well constructed in 1868 by a Mr. B. Wiley, who was one of the first settlers. (Mason, 1883, p. 313). From then to 1898 only domestic and stock. wells were constructed. Pumping for irrigation started in 1898 with the inception of the sugar-beet industry, and the first irrigation wells were at about the sites now occupied by *wells*' $10/35-25K1-10$ (pl. 1). Shortly thereafter large steam-driven centrifugal pumps, which reportedly had discharges of about 3,000 gallons a minute, were installed ·on batteries of closely-spaced wells near present wells *10j33-35Bl}* $10/34-8R1$, $10/34-19A1$, $10/35-12H1$, and $11/35-33C1$. In order to, raise the water with centrifugal pumps in the intake area, pits were dug to the water table where necessary and the pumps set on the bottom. Drifts were run out from the bottom of the pits to intercept wells drilled from the surface, and the multiple suction pipes installed. were connected to a single pump. Usually 5 to 10 wells were connected in this manner. Surface distribution was accomplished through open' ditches and flumes, and each battery of wells supplied irrigation water. to areas which were often miles away., Consequently, large "ditchlosses" resulted.

Diversion of surface water for irrigation was attempted about 1900. when water was brought through flumes and pipes from the Cuyama River to the Santa Maria plain. However, about 1908, floods reportedly destroyed the installation, and diversion from that source has not again been attempted. On the Sisquoc River similar diversion, works were installed about 1910 and are still in use (p. 63).

Until about 1920 the development of irrigation supply and increase of irrigated acreage proceeded slowly, and then in the early twenties: vegetable farming was introduced. During the next 10 years the acreage under irrigation expanded rapidly, but from 1930 to 1944 the

expansion has been somewhat slower. An intensive well-drilling program kept pace with the rapid expansion of irrigated acreage. Table 8 shows the number of irrigation wells and the approximate acreage under irrigation for the years 1920-44. The figures for years prior to 1931 were obtained from the report by Lippincott,⁸ and for the years 1931-44 were from estiniates made and factual data collected by the Geological Survey.

TABLE 8.-*Number of irrigation wells and approximate acreage irrigated in the Santa Maria Valley area,* 1920-44 .

1 From field canvass by Lippincott.
• From field canvass by Geological Survey.

'.

The table shows that in the years 1920-44 the rapid expansion of irrigated acreage went forward hand in hand with the well-drilling program. In the year 1930 and during the years 1941-44 the average number of acres irrigated by a single well was about 110. This figure was applied to the known number of irrigation wells during the period 1931 -40 to obtain estimates of acreage irrigated for those years.

The 35,000 acres under irrigation in 1944 include approximately the entire surface areas of the Santa Maria and Sisquoc plains. Of this total, about 33,000 acres, which are irrigated by nearly 300 wells, are on the Santa Maria plain; the remaining 2,000 acres, which are supplied by 17 wells, are on the Sisquoc plain. Therefore, any future development must necessarily take place on the bordering upland areas, where there are high pumping lifts, sandy soils (Watson and Smith, 1916), and somewhat less productive underlying waterbearing formations.

The yields of the irrigation wells on the plains are relatively high. Tests run on 18 selected wells by the Geological Survey showed dis-

J. B. Report on water conservation and flood control of the Santa Maria River in Santa Copy of document found at weap of the wave disposed to the second called the same in the same wand fiver in santa public at the offices of the County Planning Commission, Santa Barbara, Calif.).

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charges ranging from 400 to 1,900 gallons a minute. Similarly, tests; run on 180 irrigation wells by the San Joaqum Power Division of the· Pacific Gas & Electric Co. showed discharges ranging from 300 to. 2,200 gallons a minute. The average discharge for the 198 wells was. slightly less than 1,000 gallons a minute. The high and low yields. were about equally distributed throughout the Santa Maria and Sisquoc plains. The wide range in discharge is due to differences in. depth, perforation, and condition of the wells, and to the capacity and condition of the pumps. .

The agricultural growth of the area was accompanied by an increase. in' allied industries and in population. In addition, there was an. expansion of the oil industry following the discovery of the Santa Maria oil field in 1934. The demand upon ground water for the cattle and dairy industries, however, has remained about constant for the past 25 years.

Lippincott 9 estimated that in 1930 about 500 wells supplied water for irrigation, public-supply, industrial, d'omestic, and stock use. In. 1942 there were 311 irrigation wells, 22 public-supply wells, 20 industrial wells, and about 350 domestic and stock wells, most of which are shown on plate 1 and have been described in another report (La. Rocque, Upson, and Worts, 1950). Thus in 1942 there was a grand. total of about 700 wells that supplied water for all uses throughout. the Santa Maria Valley area.

Most of the wells penetrate the main water body for relatively short. distances. Approximately 80 percent of the 700 wells are less than 300 feet in depth, and of the remaining deeper wells only 10 are more than 500. feet in. depth. Beneath the Sisquoc· plain and the greater part of the Santa Maria plain wells derive water principally from the lower member of the alluvium, but partly from the upper part of the underlying Paso Robles formation. The few wells on the Orcutt upland derive most of their water from the lower member of the Orcutt formation, but partly from·the upper part of the Paso 'Robles formation.

Thus, the wells "skim" water from the upper part of the main water body leaving the thicker lower portion untapped. However, because water in the main body is presumed to be in hydraulic continuity: throughout both its vertical and horizontal limits, it is believed there would be no particular advantage in searching for water at greater depth. Rather, the disadvantages would doubtless outweigh the advantages for the following reasons: Drilling costs would be greater, ayaiJable data indicate that the most productive wateryielding deposits are the alluvium and locally the Orcutt formation,. which now are tapped, and it is possible that in the deepest portions.

of the main water' body the chemical quality of the water vvould be poor. The only possible advantage would be the increased yield and increased specific capacity if greater thicknesses were tapped. This would permit operation of larger pumping units which might conceivably be operated at greater efficiency than smaller units, both with respect to plant efficiency and application of water to an irrigated area or to industrial use. Doubtless this advantage would be greatest at places where the permeability of the unconsolidated deposits is · only moderate.

ESTIMATES OF PUMPAGE FOR IRRIGATION METHODS FOE ESTIMATING PUMPAGE

The methods used for estimating quantities of water pumped for irrigation are necessarily indirect because there are no water meters attached to the wells to determine directly the quantity pumped. Approximately 95 percent of the irrigation pumps in the Santa Maria Valley area are electrically operated-the remaining 5 percent are run either by tractors or stationary internal-combustion engines. Under these conditions the most accurate method of determining the pumpage was to calculate, for each of as many electrically operated plants as possible, the number of kilowatt-hours required to pump one acre-
foot of water, and to divide the average value, or energy factor, so obtained into yearly totals of kilowatt-hours consumed.

The San Joaquin Power Division of the Pacific Gas & Electric Co. kindly furnished data on more than 500 pump-efficiency tests and yearly totals of kilowatt-hours consumed for the years 1932-44. The area was divided into five subareas because of the wide.range in energy factors, which was due chiefly to the range in pumping lift. Actually, the variations in pumping lift during the period 1932-44 did not result in appreciably different average energy factors in different years. The established average energy factors range from a minimum of 130 · kilowatt-hours per acre-foot in the area of lowest lifts nears the coast to a maximum of 300 kilowatt-hours per acre-foot in the area of highest lifts on the Orcutt upland.

The yearly quantity pumped in acre-feet was determined by divid-· ing the total yearly kilowatt-hours consumed in each of the five subareas .by the appropriate average energy factor; and. the sum of the five quantities thus derived is the total amount of water pumped each year from the entire area by electrically operated pumps. This total was then increased by 5 percent, to allow for the quantity pumped by nonelectrically operated pumps, to obtain the yearly total pumped for irrigation by both classes of pumps for the period 1932-44.

The quantity pumped each year prior to 1932 had to be determined $\frac{1}{2}$ by a second method which is based on the yearly totals derived above.

^{*} Lippincott, J. B., op. cit., p. 23.

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It was found that during the period $1932-44$ the average yearly depth of water pumped onto the irrigated land, or the duty of water, varied approximately in accordance with rainfall. During years of above a verage rainfall the duty of water was about 1.7 acre-feet per acre. whereas during years of below-average rainfall the duty of water was. about 2.1 acre-feet per acre; thus, the average duty was about 1.9' acre-feet per acre for the whole period. Yearly rainfall was plotted. against the duty of water for the years $1932-44$, and a smooth curvewas drawn through the points. For years prior to 1932 values of rainfall were then plotted on this curve and corresponding values for duty of water obtained. These values in turn were multiplied by theknown or estimated irrigated acreage to determine yearly pumpage. In this manner it was possible to estimate the pumpage by years as. far back as the acreage irrigated was known (table 8). The total quantity pumped for irrigation for the years 1929-44 is shown in. table 9. The return of IRRIGATION WATER

The total quantity of water pumped for irrigation as computed above is not the quantity permanently removed from storage. In the intake area a part of the total quantity pumped each year for inrigation seeps below the root zone and returns to storage in much the same manner as does the infiltration of rain. The greater part, however, is lost by transpiration and evaporation.

The quantity of irrigation water which returns to storage each year varies considerably from one part of the area to another, depending primarily on type of soil, type of crop, irrigation practice, and climatic conditions. It is probably greatest in the Sisquoc and upper Santa Maria valleys where the soil is sandy. 'Westward, down the Santa. Maria Valley the soil is heavier and less water returns to storage. In the area of confinement (pI. 5), which includes approximately one-third of the irrigated area, little or no return occurs, and essentially all waterin excess of that transpired or evaporated eventually discharges from the shallow water body into the sea $(p. 74)$.

The amount of irrigation water which returns to storage each yearin the intake area (about two-thirds of the irrigated area) ranges from essentially no return along the inland boundary of the area of confined water to possibly as much as 50 .to 60 percent of the yearlypumpage in the eastern part.¹⁰ Thus, the average return in the intake area is estimated to be about 30 percent of the pumpage, and. for the entire area, including the area of confinement in which there is. little or no return, about 20 percent of the total pumpage. Therefore,. of the total quantity of water pumped for irrigation each year, an estimated 80 percent is permanently removed from storageent puns at with NoNewWipTax.com

 10 Based on data compiled for other coastal areas of California by Harold. Conkling.

quantity is designated the total net pumpage. Table 9 shows both the total pumpage and the total net pumpage for irrigation for the-16-year period 1929-44.

'TABLE 9.-Estimates of pumpage for irrigation from the main water body, 1929-44

Pumpage during 1940 was the greatest on record and was due in large part to the relatively low and poorly distributed rainfall during that year. Pumpage declined in the early thirties, probably owing mainly to the economic conditions which prevailed at that time, but has increased steadily since then. Presumably additional lands will beplaced under irrigation in the future. Lippincott¹¹ has estimated that there are $50,000$ acres of irrigable land in the area-an excess of 15,000 acres or 40 percent above that now in use. If all this land were placed under irrigation, or if more double-cropping were practiced on the present acreage, and the pumpage were increased proportionately, the total net pumpage for irrigation alone would be more than 80,000 acre-feet a year.

ESTIMATES OF DISCHARGE FOR USES OTHER THAN IRRIGATION

Pumpage for uses other than irrigation includes withdrawal for public-supply, industrial, domestic, and stock uses. Also included is the flow from artesian wells, which is artificial discharge. The methods. used for estimating each differ according to the available data, and are discussed separately below.

PUBLIC SUPPLY AND DOMESTIC USES

The largest single use for public supply is that for the city of Santa Maria from three wells on the Orcutt upland. Fortunately, thewater pumped from these wells is metered and the yearly pumpage ¹¹ Lippincott, J. B., op. cit., p. 16, 1931.

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can be obtained directly. Records of municipal use have been made available by the City Water Department. In 1944 the pumpage was about 1,700 acre-feet.

As nearly as can be determined, the population of the outlying towns and rural areas was about 4,500 in 1930, and about 5,100 in 1940 (Bureau of the Census 1940).¹² The quantity consumed in these areas is based on an estimated per capita use of 125 gallons a day. This quantity allows for gardening, for use by small-business establishments in the smaller towns, etc., Thus, the quantity pumped for the rural population is estimated to have been about 670 acre-feet in 1930, and about 760 acre-feet in 1940. For the intervening and subsequent vears the average vearly increase is apportioned.

INDUSTRIAL USE

The estimate of pumpage for industrial use is based on the reported and inferred capacities of the pumps and their operating schedules. The principal industrial uses are for ice plants, packing sheds (excluding those in Santa Maria, which are supplied by the city wells), and oil refineries. Most of these plants operate only during the day. A total of 20 industrial wells were active in 1942. Each of these wells was visited and from the data gathered at that time it is estimated that the average daily schedule was about 5 continuous pumping hours, and that the average yield of each well was approximately 500 gallons a minute. Thus, in 1942 the yearly pumpage by the 20 wells for industrial use is estimated to have been about 3,500 acre-feet.

For years prior to and after 1942 no data are available on the exact number of pumping plants in use by industries. However, it is believed that the pumpage has increased steadily since 1929. Because no reliable data are available, and further because the estimate of pumpage in 1942 is only approximate, it is assumed that the pumpage has increased at a rate of about 100 acre-feet a year. Thus, in 1929 the pumpage for industrial use may have been about 2,000 acre-feet.

STOCK USE

It was reported ¹³ that for the past 25 years there has been an average of about 7,000 head of dairy cattle in the Santa Maria Valley area, and the quantity of water required per head is ordinarily estimated as 15 gallons a day. In addition, 15 gallons a day per head is required for dairy operation and maintenance. Thus, the averages pumpage for stock use has been about 200,000 gallons a day, or roughly 250 acre-feet a year.

¹² Also data from Santa Maria Valley Chamber of Commerce. ¹³ Eriksen, H. C., personal communication, Nov. 1945.

FLOW FROM WELLS

Ground-water discharge by flow from wells is considered artificial discharge. Most of these wells are allowed to flow unchecked, and in that sense the water discharged is a needless waste of ground water. During the 4-year period 1942-45, the discharge from artesian wells amounted to nearly 2 percent of the total discharge by pumpage. Although relatively small, this amount is worth conserving.

In 1942, there were 20 flowing wells distributed over about 5 square miles of the arable part of the area of confinement (pls. 1 and 5). The quantity of flow from each was estimated in the spring of that year by the Geological Survey, and it was found that the total discharge amounted to approximately 1.250 gallons a minute. Most of these wells were revisited at various seasons of the year during the 4-year period 1942–45, and it was found that the flow decreased substantially during the summer pumping season, but increased to about the same discharge each spring. The average flow is estimated to have been about 700 gallons a minute, or 1,200 acre-feet a year.

Lippincott¹⁴ reported that the area of flow was 23 square miles in 1918 and extended eastward almost 1 mile from Guadalupe (pl. 5). but in 1930 the area of flow has decreased to 1.5 square miles. Records of water levels indicate that by 1936 the area of flow was even less. During this time the discharge by flow from wells varied according to the head and to the number of wells permitted to flow unchecked. It is estimated that the flow decreased from an unknown maximum in 1918 to a minimum in 1936 of about 300 gallons a minute, or 500 acre-feet a year, and that in the years 1937-42 the flow increased steadily with the increase in head. The yearly quantities of flow during the years 1929-44 are apportioned according to the rough estimates derived above.

ESTIMATE OF TOTAL DISCHARGE FOR OTHER USES

The total discharge for nonirrigation uses (as described in preceding pages) amounts to only about 10 percent of the total net pumpage for all uses. In view of this relatively small percentage, the estimates may be in error without affecting appreciably the estimate of total discharge from the main water body. Accordingly, more refined estimates of pumpage for minor uses are not considered justifiable at this time.

Table 10 shows the total yearly pumpage for public-supply, industrial, domestic, and stock uses, and includes the discharge by flow from wells-all of which are designated discharge for use other than irrigation.

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TABLE 10.-*Estimates of total yearly discharge from the main water body for use* other than irrigation, 1929-44

This discharge for nonirrigation uses remained about constant during the early and middle thirties, probably owing to economic conditions and to the decrease in flow from' wells. In the later years, and during the war years 1941-44 in particular, pumpage increased . rapidly to keep pace with the wartime population. It is not believed that the increase will continue at the present rate in postwar years, but there is every indication that pumpage will remain above its prewar leveL

NATURAL DISCHARGE

. FORMS AND AREAS OF DISCHARGE

Natural discharge of ground water is all discharge other than pump~' . age and artesian flow from wells. It includes outflow or submarine discharge into the sea, overflow into streams at the eastern edge of the area of confinement and thence to the sea, and evapotranspiration by native vegetation where the water table is close to the land surface. Prior to the drilling of wells, all recharge to the main water body in excess of that retained in storage was dissipated by natural discharge.

Water moving seaward beneath the confining beds at the west end of the Santa Maria Valley is discharged from the main water body into the Pacific Ocean through the unconsolidated deposits exposed on the ocean floor. The confining beds composing the upper member, of the alluvium extend off shore for a distance of about 2 to 4 miles; causing water to be discharged west of that boundary $(p.78)$. Water thus discharged from the main water body is designated as natural discharge by ground-water outflow. Since the mid-twenties natural discharge has taken place only in this form.

During consecutive years of excessive recharge prior to the advent. of heavy pumpage in the mid-twenties, outflow could not dispose, the large ground-water increment. Oonsequently, the water table rose in the intake area until water flowed over the eastern edge of the

confining beds (pl. 5). It is reported by several residents that from about 1914 to the mid-twenties perennial surface flows occurred in Green Canyon, starting near well 10/35-13J1; and in the Santa Maria River, starting several miles east of the railroad bridge. Water thus discharged from the main water body is designated as natural dis-
charge by ground-water overflow.

Evaporation and transpiration losses occurred only at times when and in the areas where overflow was taking place. It is reported that there were heavy growths of water-loving plants in the channels downstream from the areas of overflow in the late teens and early twenties. Doubtless, fairly large evapotranspiration losses took place in these reaches. Elsewhere, the water table has remained below the reach of water-loving plants. The absence of plants and trees of this type in the intake area along the Sisquoc and Santa Maria Rivers bears. evidence to this fact. Furthermore, it is reported by residents that the Santa Maria and Sisquoc plains and river channels have always been barren of this type of vegetation. Thus, losses from the main water body by evaporation and transpiration are practically nonexistent, although a shallow water body (p. 74) supports plant growth at the west end of the valley.

ESTIMATES OF DISCHARGE BY GROUND-WATER OUTFLOW

. The method used to compute the quantity of ground-water discharge by outflow to the sea is based on Darcy's law (p. 76). Therefore, it is necessary to know the saturated cross-sectional area of the water-bearing formations at or near the coast, the permeability of each, and the slope of the pressure surface, or hydraulic gradient.

Geologic section $D-D'$ (pl. 2) shows the cross-sectional area of the deposits through which the water being discharged at the coast must move. These formations are the Oareaga sand, the Paso Robles formation, the lower part of the Orcutt formation, and the lower member of the alluvium. The cross-sectional area of the lower member of the alluvium is determined from numerous water-well logs and, therefore, is fairly accurate. The cross-sectional areas of the other formations are only roughly defined by a few data from oil tests. Furthermore, the area of outflow is limited on the north by the ground-water divide, as water moving north of that divide is not part of the Santa Maria Valley area discharge.

With respect to the saturated cross section along line $D-D'$, it may be noted that beneath the Santa Maria plain all water-bearing deposits are confined beneath the upper member of the alluvium and their entire section is saturated-including the lower member of the alluvium, the Orcutt (?) and Paso Robles formations, and the Careaga \sim sand water unthe relatively small areas of unconfined water north and

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south of the plain, the water table fluctuates from year' to year and so changes the saturated cross section. Because these fluctuations are negligible when compared to the very large saturated section, the saturated section throughout the length of section $D-D'$ is considered t_0 be constant.

. As thus defined, the cross-sectional area of the saturated portion of the Careaga sand is approximately 11,800,000 square feet, of the Paso Robles and Orcutt formations about 29,200,000 square feet, and of the lower member of the alluvium about 2,238,000 square feet; or a total saturated, cross-sectional area of somewhat more than 43,000,000 square feet.

The permeabilities of the various formations, which are in part estimated, have been discussed elsewhere, and the coefficients applied to this cross section are as follows: for the Oareaga sapd about 75 gallons per day per square foot---the laboratory permeability of 70 at 60° F. (p. 28) adjusted to field temperature of 64° F, by dividing by the conversion factor 0.95 (Wenzel, 1942, p. 62); for the Paso Robles and the Orcutt formations about 65 gallons per day per square foot $(p. 33)$; and for the lower member of the alluvium about 2,000 gallons" per day per square foot (p, 39).

The hydraulic gradient, although relatively slight at the line of section, has varied considerably from 1918 to 1944. Plate 6 shows water-level profiles for the main water body for 1907, 1918, 1936, and 1944. The profiles for 1907 and 1918 were obtained from the report by Lippincott,¹⁵ and those for 1936 and 1944 were compiled from data. collected by the Geological Survey. The profile for 1936 is the lowest of record and that for 1918 is the highest. The profiles in 1907 and 1944 are intermediate and are nearly coincident. All four have been; projected to the coast line to show roughly how the head of water, has varied at that place. A maximum head about 55 feet above sealevel occurred in 1918, and a minimum head of about 20 feet in 1936. On plate 6 the hydraulic gradients at the crossing of section $D-D'$ appear to have varied only slightly in the 4 years. However, on profiles of larger scale it was found that in the 3 years 1918, 1936, and 1944 the hydraulic gradients were 10, 6, and 8 feet per mile, respectively. Owing to the irregularity in the gradient in 1907 near, the line of section, this year is omitted from the outflow computations. However, outflow was probably of about the same magnitude as in

1944. Because the water in the main water body is considered to be essentially confluent throughout, the hydraulic gradients then, are applicable to the full cross-sectional area. Thus, the total ground-, Copy of document found a

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water outflow to the sea for the 3 years, 1918 , 1936 , and 1944 , or for other years when the hydraulic gradient is known, can be estimated as follows: The rate of ground-water outflow in gallons per day is equal to the product of the cross-sectional area times the permeability times the hydraulic gradient. The rate times 365 gives the total quantity for any 1 year. The quantities thus derived are shown in table 11 for the maximum discharge in 1918, for the minimum discharge in 1936, and for the discharge in 1944.

TABLE 11. - *Estimates of ground-water outflow from the main water body in 1918*, " 1936, *and* 1944 " '

The table shows that there has been a fairly wide range in groundwater outflow to the sea. Furthermore, approximately two-thirds of withe total outflow takes place through the lower member of the alluvium, which constitutes only about 6 percent of the total cross-sectional area.

It is believed that in the years around 1918, when water levels were NaNew Wip Lax.com
"the highest of record and when ground water was discharging along

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the edge of the area of confinement as overflow in Green Canyon and the Santa Maria River, approximately the maximum possible hydraulic gradient was established across the area of confined water. Hence, the discharge of 16,000 acre-feet a year is also the maximum. possible ground-water outflow. This is believed to be true because any further increase in gradient in the intake area would produce an increased overflow, but it would increase only slightly the hydraulic. gradient in the area of confinement. Hence, the outflow could not be increased appreciably, .

In 1936, on the other hand, when the minimum known hydraulic gradient of 6 feet per mile occurred, the outflow also was at a minimum, or 9,500 acre-feet. The overflow had long since ceased, and water levels along the edge of the area of confined water had declined 55 feet. The pressure head at the coast had also declined from a projected high in 1918 of about 55 feet above sea level to a projected low in 1936 of about 20 feet (pI. 6). Thus, if the water levels had continued to decline after 1936, the hydraulic gradient would have decreased accordingly. Ultimately, when the hydraulic gradient approached zero outflow would also approach zero. In order for this to happen, the water levels along the eastern edge of the area of confined water would have to'be reduced nearly to sea level, or about 80 feet below the. 1936 levels; however, this possibility was averted when water levels· began to rise in 1937.

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> Thus, ground-water outflow has ranged from a maximum of about $16,000$ acre-feet in 1918 to a minimum of about 9,500 acre-feet in 1936. With the subsequent rise in water levels and increase in hydraulic gradient trom 1936 to 1944 it has increased to nearly 13,000 acre-feet a, year. For the intervening years not shown in the table, outflow has been estimated for years when there were sufficient water-level data and interpolated for the remaining years. The total outflow for the 16 -year period 1929-44 is estimated to have been about 18.0,000 acrefeet, or to have averaged slightly more than 11,000 acre-feet a year:

ESTIMATES OF DISCHARGE *BY* GROUND-WATER OVERFLOW AND EVAPOTRANSPIRATION

Discharge by ground-water overflow can be very roughly estimated for the years around 1918. These estimates include any evapotranspiration losses that may have occurred at that time. During the 14year period 1905-18, rainfall was above average, and as a result a considerable quantity of recharge was supplied, to the main water' body, Recharge undoubtedly was greater than during the years of below-average rainfall, 1929-36, but probably was less than that in the extremely wet years, 1937-44 (p. 83). If an average recharge of about 80,000 acre-feet is assumed as reasonable for the period 1905-18 (p. 126), then the distribution of discharge for the years will period 1905-18 (p. 126), then the distribution of discharge for the years when ϵ around 1918 can be roughly annroximated as follows: Discharge by

outflow amounted to about 16,000 acre-feet a year (table 11), and pumpage was possibly about 20,000 to 30,000 acre-feet a year. Thus, discharge by overflow and evapotranspiration losses must have made up the difference and would have been about 30,000 to 40,000 acrefeet a year.

Perennial flows in Green Canyon and in the Santa Maria River totaling approximately 40 to 50 second-feet would have been necessary to dispose of the excess recharge. As was mentioned, flows in both channels were reported by residents, but unfortunately the magnitude of the flows was not known by them. If such large yearly flows were discharged at the surface, a considerable portion would have been lost by evaporation and transpiration (p. 93). However, the discharge by overflow and evapotranspiration cannot be divided.

Overflow and evapotranspiration losses have not taken place since the depression of the water table in the mid-twenties. Therefore, no estimates are included in table 12. It is certain that neither form of discharge will recur as long as the water table remains at or near the levels induced by the heavy pumpage of the past 20 years. Thus, .in effect, pumpage has salvaged a large amount of natural discharge for agricultural and other uses which otherwise would have been lost.

ESTIMATE OF TOTAL DISCHARGE

The estimate of total discharge from the main water body includes the discharges by net pumpage for irrigation, discharge for other 'uses, and ground-water outflow to the sea (tables 9, 10, and 11, and p. 96). These data have been assembled for the 16-year period 1929-44, and are shown in table 12.

Total Total **Year** Year discharge discharge
(acre-feet) (acre-feet) 1030 $^{10.99}$.
1930 __________________
1930 __________________ 57, 500 1940 ___ ~_~~ ____ ~ ___ ---~- I941 _____________ ~_~ ___ _ 1942 ___ ~ ______ . _________ _ 1943 ______________________ _ 1944 ___________________ _ 69, 800 77, 800 65, 300 58,900 1931. 59, 400 68, 600 1932. 56, 900 ;!~:J~i~==== = = = == = = == == == === '" 1934 _________________ ~~_ 51, 600 74, 500 53;600 78, 000 ::: 1934----------------------|
/:: 1095 56, 100 62, 800 $\frac{1000}{2000}$ 1936 ~. 1937 __ ~ ________________ _ Total____________ 1, 016, 200
16-year average____ 63, 500 62, 100 16 -year average___ 1938 -------------63, 300 $\sum_{i=1}^N \sum_{j=1}^N$

 $E_{\rm{c}}$. Total yearly discharge, which probably increased steadily during the twenties, reached a peak in 1931, decreased slightly to 1933, then \generally paincreased steadily to 1944. Thus, the minimum of record was about 51,600 acre-feet, the maximum was about 78,000 scre-feet.

TABLE 12,-Estimates of total yearly discharge from the main water body, 1929-44

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and the 16-year average was about 63,500 acre-feet. The relatively high discharges in 1939 and 1940 were due in part to the relatively low rainfall and consequent increase in net pumpage for irrigation. Because the net pumpage for irrigation has composed about 75 percent of the total discharge in recent years, the variations in pumpage have been the principal cause of the variations in the total discharge.

WATER-LEVEL FLUCTUATIONS IN THE MAIN WATER BODY

SCOPE AND UTILITY OF THE RECORDS

In the Santa Maria Valley area six agencies have made over 4,500 depth-to-water measurements in 71 observation wells. The agencies and their span of record to date are the city of Santa Maria, beginning September 1917; the Union Oil Co. of California, beginning March 1920; the San Joaquin Power Division of the Pacific Gas and Electric Co., beginning August 1929; J. B. Lippincott, a single set of measurements in September and October 1930; the Santa Maria Valley Water Conservation District, beginning April 1938; and the United States Geological Survey, beginning May 1941. In addition, there are fraginentary records by owners, well drillers, and pump agents. All these records have been assembled and released to the public in published reports by the Geological Survey (La Rocque, Upson and Worts, 1950. Meinzer, Wenzel, and others, 1943, pp. 147-153; 1944, pp. 228-237; 1945, pp. 177-183. Sayre, A. N., and others, 1947, pp. 156-163; 1949, pp. 168-175).

In any area, records of water-level fluctuations in wells are of inestimable value to the hydrologist for the interpretation of the past and present hydrologic conditions. The records collected in the Santa Maria Valley area showed several types of fluctuations pertaining to the conditions or forces at work in the main water body, as follows: recharge from streams, recharge from rain, pumping, and moving load on the land surface. The first three types may be cyclic and commonly produce a yearly or seasonal effect-differences among which, in part, serve to identify the cause. The fourth operates momentarily and has no large effects. The several types of fluctuations are discussed in the ensuing pages.

Figures 3, 4, 5, and 6 show fluctuations of water level in 13 selected wells in the Santa Maria Valley area. The hydrographs of the wells shown on figures 3, 4, and 5 are in the intake area, and those on figure 6 are in the area of confined water. The locations of these wells are shown on plate 1.

the intake wells adjacent water levels in three ် $-F$ luctuations 99

 001 7935 1936 1937 1938 1939 $\frac{1}{1930}$ $\frac{1}{1931}$ $\frac{1}{1932}$ 1933 $\frac{1}{1934}$ 1940 -1929 1941 1942 1943 1944 60 GEOLOGY AND GROUND-WATER, SANTA MARIA VALLEY, CALLY, \bar{z} $\overline{7}$ 6^c 10/34-2RI (Altitude 230 feet) 9 LAND SURFACE DATUM 100 $\overline{1}$ (2) $\overline{\mathbf{3}}$ $\overline{4}$ ///34-30 0/ (Altitude 148 feet) WATER LEVEL, IN FEET, BELOW 50 $_{\rm sc}$ \overline{v} $_{\rm gc}$ $^{90}_{70}$ \ddot{a} 10/33-1981 (Altitude 275 feet) ່ 9 10 $\bar{\mathbf{u}}$ 121 FIGURE 4.-Fluctuations of water levels in three wells in the intake area, 1929-44. <u> Anglick Communication</u> .
Matamatan الكائد المتفاضح 978
14 9230 रम्पु 50 1917 1918 1919 1920 1921 1922 1923 1924 1925 1926 1927 1928 1929 1930 1931 1932 1933 1934 1935 1936 1937 1938 1939 1940 1941 1942 1943 1944 60 $10/34 - 1453$ (Allitude a
225 feet) \overline{z} -áo **DATUM** 90 lor $\frac{SURFACE}{R}$ GROUND-WATER REEOGROURCE $rac{3}{5}$ 130 140 PEET, BELOW 51 6^o $9/32 - 7N2$ (Allitude 430 feet) \mathbf{z} $\overline{\mathbf{z}}$ \mathbf{d} $\overline{\mathbf{g}}$ ĝ 9 ÷ (9) 120 L. 130 $9/34 - 3N3$ (Allitude 253 feet) 140 150 160

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FIGURE 5.--Fluctuations of water levels in three wells in the intake area, 1917-44.

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FLUCTUATIONS CAUSED BY RECHARGE FROM STREAMS

Because the water table nearly everywhere is far below the channels of the Sisquoc and Santa Maria Rivers, seldom, if ever, is there any hydraulic continuity between water in the channels and the main water body (p. 79). Thus, the levels in wells always rise in response to recharge from the two rivers but have never been known to fluctuate. in accord with river stage.

Stream flow, and hence recharge from the rivers, commonly is limited to the 6-month period November through April each year; and during the remaining 6 months channels of both the Santa Maria and lower Sisquoc Rivers are usually dry. Accordingly, the response of water levels to stream recharge is cyclic in nature. Furthermore. the magnitude of the rise each year is dependent upon the quantity. of recharge. During years of low recharge small rises occur and, of course, the converse is also true.

In the intake area the rise is due directly to the increase in stored water and represents actual saturation of the deposits, whereas in the area of confinement the rise is due solely to the increase in head in the adjacent intake area. Also, both in the intake area at some distance from streams and in the area of confinement there is a considerable time lag in the response of water levels to river recharge. Hence, the fluctuations in each are discussed separately.

Fluctuations in the intake area.—Wells along the river in the intake area are the first to respond to each year's recharge, and without exception they have larger rises than those in any other part of the area. The hydrographs of wells 9/32-7N1, 9/33-2A1, 10/33-28A1, $10/34-2R1$, and $11/34-30Q1$ (figs. 3 and 4) show the character of the rises. They show that during years of substantial recharge there is a steady, uninterrupted rise of water levels during the winter months of each year, and that the net rise is roughly proportional to the quantity of recharge. For example, in 1941, the year of greatest recorded recharge (table 7), the hydrographs of wells $9/32-7N1$, $9/33-2A1$. 10/33-28A1, 10/34-2R1, and 11/34-30Q1 show net rises of about 25, 20, 35, 35, and 30 feet, respectively. A relatively small and indeterminate part of these rises was due to infiltration of rain and recovery of water levels at the close of the summer pumping season. On the other hand, in 1940, a year of relatively small recharge, the hydrographs of wells 2A1, 28A1, and 11/39-30Q1 show net rises of only about 3 feet, and wells $9/32-7N1$ and $10/34-2R1$ show slight net declines. The net declines were due either to the fact that the depletion of storage by natural drainage exceeded the recharge, or to local pumpage, or both.

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In wells progressively farther away from the river, the water levels respond to recharge from streams in much the same manner, but the rises are progressively smaller in magnitude, occur at slower rates, and are interrupted or partially masked by summer pumpage. These featmes aie shovm. most clearly by wells 10/33-19Bl and 10/34-14E3 (figs. 4 and 5), which are more than a mile from the river. In 1941 the water levels in these wells rose.24 and 18 feet, respectively, which was somewhat less than those near the river; did not reach their peaks until 6 to 12 months after the levels in wells near the river; and rose at rates of 3 and 2 feet per month, respectively, for 8 months compared to an average rate of rise of 8 feet per month for 4 months in wells near the river.

Thus, each year's recharge travels away from the river as a mound: The mound probably registers in large part the transmission of head . rather than the actual movement of water (Tolman, 1937, p. 241). However, in this report it is referred to simply as the recharge mound because in either case the effect on water levels is essentially the same. Water-level contour maps for 1941 and 1942, drawn for study purposes, show that, the recharge mound which developed from river seepage loss in 1941 moved southwestward away from the Santa Maria River and decreased in height as it traveled. The mound took from 6 to more than 12 months to reach the southern edge. of the Santa Maria plain. It decreased considerably in height and in volume by the time it reached wells 3 or 4 miles from the river, but ultimately it may have extended as far south as the axis of the ground-water trough beneath the southern part of Orcutt upland, as shown for the spring 1942 (pl. 5). However, its effect on water levels in this area was probably masked by infiltration of rain.

During years of average recharge from streams the mound probably does not extend far beyond the southern edge of the plain, and during years of below~average recharge it probably does not move even that far south.

Fluctuations in the area of confined water.--An increase in groundwater storage by recharge from streams in the intake area, as was explained above, results in a rise of water levels. This rise increases the head of water in the underlying formations, which farther west are confined beneath the upper member of the alluvium. This increase in head, in turn, is reflected by a rise of the water levels in: wells. However, because the boundary between the two areas is gradational, and because, as was indicated previously, there is a considerable time lag in the movement of the stream-recharge mound southward across the plain, this transmission of head is a slow process.

These relationships are best illustrated by a copy parison of the α which ex wip α and hydrographs of wells $11/34-30Q1$ (fig. 4) in the intake area and $10/35-$

 $12M1$ (fig. 6) about a mile within the area of confined water. In well 10/35-12M1 the principal rise after the 1941 recharge did not culminate until the winter of 1942, and thereafter the annual peaks rose slightly to 1944; whereas in well 11/34-30Q1 the peaks declined slightly from that of 1941 ..

The lack of pronounced rise in well $10/35-12M1$ in 1941, in response to the large recharge of that year, is probably explained by the fact that the recharge mound moving southward from the river in the intake area did not reach the inland edge of confinement east of this well until mid-summer, and that it was dampened by summer puinpage.

The rise in well 10/35-12M1 from October 1941 to April 1942, which amounted to over 10 feet, cannot be traced to recharge in the winter of $1941-42$, because the hydrograph of well $11/34-30Q1$ shows no appreciable rise in the spring of 1942. Thus, the rise of water level in well'10/35-12M1 in 1942 must have been due primarily to the general rise in the water table in the intake .area resulting from the large recharge in 1941. In well $11/34-30Q1$ even the summer levels in 1942 and in later years were about 15 feet higher than in preceding years, and this rise is somewhat greater than but similar to the rise in the range of fluctuations in well $12M1$. The hydrographs of wells 10/35-7F1, 11/35-20E1, and 11/35-33G1 (fig. 6) all show the same features. The peaks to which the water levels might have risen in wells 7F1 and 20E1 could not be ascertained because both flowed for several months in the winters of 1942-44 and measurements of static head 'were not made. The peaks were probably somewhat lower than in well $12M1$.

Therefore, it is concluded that a recharge mound leaving the river . in the intake area does not affect the head of water in the area of confinement until it has produced a general rise in water levels along · the greater part of the inland boundary of confinement. The time lag involved is from 6 months to a year, and again the amplitude of rise is directly proportional to the quantity of recharge. Thus, even in the area of confinement the effect of river recharge decreases as the distance of the wells from the area of river recharge increases.

FLUCTUATIONS RELATED TO RECHARGE FROM RAIN

The relatively small quantities of recharge by infiltration of rain usually cannot be identified in the hydrographs of wells in most of the intake area because the' response of water levels to rain is overshadowed by other larger responses, such as those to recharge from streams and to recovery from pumping. Nevertheless, in each season's rise of water levels in years when the rainfall is greater than about 12 which es, with the some small increment that is due solely to rain. . In areas remote from heavy pumping and from river recharge it is

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believed that recharge from rain can be' recognized. Furthermore, there appears to be some time lag involved between the time of rainfall· and the time the recharge reaches the main water body.' For example, in the year 1942 the water level in well *9j34-3N3* (fig. 5) rose about 6 feet. Assuming a specific yield of about 16 percent (p. 119), and recharge from rain in 1941 as about 1 acre-foot per acre (table 6), the rise of water level from that recharge alone would have been about 6 feet. Thus, it is believed that the rise was due largely to rainfall and . perhaps partly to the decrease in pumping rate, Conceivably, a part of the rise may have been due to river recharge in 1941. The quality of water in this area indicates, however, that recharge is primarily from the in6ltration of rain' (p. 137).

Isolated measurements in wells $9/33-15D1$ and $11/34-19R1$ show net rises from the mid-thirties to 1944 of about 15 and 6 feet, respectively-a rise which is believed to be due primarily to recharge from rain. In the minor water bodies in the Orcutt and Nipomo uplands $(p. 74)$ water levels are reported to rise in years of above-average rainfall and to show little or no rise in years of below-average rainfall.

FLUCTUATIONS INDUCED BY PUMPING

Pumping of ground water for all uses in the Santa Maria Value area has a considerable diurnal as well as seasonal fluctuation-most. pumping being in the daytime, and most of it in the summer. The resulting variations in draft on ground water produce daily and seasonal fluctuations of water levels in wells. Because pumpage for irrigation constitutes the bulk of the draft its effect is the most pronounced. The length of the pumping season is dependent to a large degree upon the distribution and intensity of rainfall, but in general about 90 percent of the pumping occurs during the 7-month period April through October; the remaining 10 percent takes place during the winter months. 15

Seasonal fluctuations.-In contrast to the general seasonal rise of water levels in wells due to recharge each year, there is a corresponding seasonal decline of water levels due mostly to discharge by pumpage during the late spring, summer, and early autumn months. However, an undetermined part of each year's decline is due to the continuous' process of natural depletion of storage by the westward drainage of. ground water. As a result of the staggered periods of yearly recharge and discharge the hydrographs of wells show an oscillation somewhat analogous to a sine curve. Late each spring water levels in most wells begin to decline abruptly as pumping for irrigation begins; generally at about the same time throughout the area. Naturally, the response

¹⁸ Lippincott, J.B., Report on water conservation and flood control of the Santa Maria River in Santa Lippincott, J. B., Report on Water conservation and hood control of discomposition in the published in the published in the published in the published at with NoNewWipTax.com
Barbara and San Luis Obispo Counties, Calif., M the offices of the County Planning Commission, Santa Barbara, Calif.).

is most noticeable in wells in the heavily-pumped areas of the Sisquoc and Santa Maria plains. . Elsewhere the response varies principally with the distance of wells from these heavily pumped areas.

. In the intake area water levels in wells adjacent to the rivers usually closely approach or occasionally reach their peaks before,pumping for . irrigation begins each year. Consequently, in these wells the decline of water levels due to pumping is not appreciably masked by river recharge as in the case of water levels in wells farther away. During the 4-year period 1941-44, when monthly measurements were made in most observation wells, the hydrographs of wells $9/32-7N1$, $9/33-2A1$, 10/33-28Al; *10j34-2Rl,* and llj34~30Q1 (figs. 3 and 4) showed declines each year which averaged about 7, 8, 13, 11, and 9 feet, respectively. Most of these wells show that the water levels reach their lowest stages near the end of each year and sometimes*not until January or February of the following year. In years of small recharge, such as 1939, the hydrographs show that in general water levels continued to decline after February and throughout the remainder of the year.

In the intake area away from the river the decline of water levels each year in response to pumpage is greatly dampened or is even nullified by the delayed recharge mound from the river. . The hydrographs of wells 10/33-19B1 and 10/34-14E3 (figs. 4 and 5) show that in. 1941, instead of declining, water levels rose rapidly from about April throughout the period of concentrated pumping. This same characteristic was noted in other wells in the same area, and also for other years of large recharge, such as 1938. In fact, water levels in some wells in this area are occasionally at their lowest stage in February and March, when wells along the river are approaching their peaks. This is shown by a comparison of the hydrographs of wells 1033-19B1 and 10/34-2R1 in the years 1943 and 1944. 'Wells along the south side of the plain show normal spring rises and summer declines only because the recharge mounds reach this area almost 1 year late and, therefore, do not mask the pumping decline.

In the heavily pumped portion of the Orcutt upland fluctuations of water levels in response to pumping are different from those elsewhere in the area. . This is due to the fact that wells $9/34-3N1$, $9/34-3N2$, 9/34-3N3, 9/34-3N4, 9/34-10M1, and 9/34-10M2 are all publicsupply wells and are necessarily operated during the entire year. Because summer pumpage is greater than winter pumpage, water levels show some variation. The fluctuations induced by pumping are best illustrated by the hydrograph of well $9/34-3N3$ (fig. 5) which, except during winters of large recharge, shows exceedingly small variations in water levels between winter and summer, amounting to only $1 \text{ to } 3 \text{ feet.}$
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In the area of confined water, on the other hand, water levels start to decline instantly when pumping begins, usually in April, reach their IO'west stages at the height of the pumping season in July) August, or September, and start to rise rapidly thereafter as pumping decreases. The immediate rise or decline of water levels in response to pumping conditions is due primarily to the fact that the fluctuations largely represent changes in head, and not the unwatering of deposits as is the case in the intake area. When the pressure is reduced at the start of the pumping season, the loss of head throughout the artesian system is rapid, and water levels drop.⁻ At the close of the pumping season just the reverse takes place.

The close correlation between pumping schedules and water-level fluctuations in the area of confinement is best shown by the hydrographs of wells 10/35-7F1, 10/35-12M1, 11/35-20E1, and 11/35-33G1 $(f_{\text{f},f}, \theta)$. These hydrographs show clearly the start, height, and termi-. nation of the pumping season as outlined above. During the 4-year period 1941-44 the seasonal drop in water levels has averaged about . 8 feet each year. This uniform amount of. seasonal decline may be due to the fact that pumpage during each of these four years has been of about the same intensity and duration.

Diurnal fluctuations.-The diurnal fluctuations of water levels in . response to pumping in the intake area differ considerably from those in the area of confinement. The records from recorder charts and float gages of well 10/33-27K1 in the intake area and of well 10/35-7G3 in the area of confinement are compared to show the effects of pumping in the two areas on the daily fluctuations.

Well $10/33-27K1$ is about 300 feet from irrigation well $10/33 27K2$. During each day of a 150-day period from April 27 to September 24, 1942, the water level in well 27K1 dropped almost consistently about 0.15 foot in response to pumping in well 27K2, and recovered about 0.05 foot during the night after the pump shut down, for a total net decline of 12.61 feet during the entire period. Furthermore, there was a lag of several hours between the time pumping started and stopped in well 27K2 and the time when the .water level in well 27K1 responded.

. In the area of confinement', on the other hand, the record of well $10/35-7G3$ shows an entirely different response to daily pumping in iriigation well 10/35-7G1, which is only 250 feet distant. Well 7G3 shows a daily decline of over 6 feet and a nocturnal recovery of almost the same magnitude. From May through September 1942, the net decline was only 7 feet, indicating that diurnal fluctuations due to pumping were often as great as the total seasonal fluctuation. Also, when irrigation well 10/35-7G1 was started and stopped the test manuschia at www.noNewWipTax.com in observation well 7G3 was abrupt and almost instantaneous.

Another feature illustrated by comparing hydrographs for wells in these two areas are the diurnal fluctuations produced in the observation wells by pumping in distant wells. In well $10/33-27K1$ only a barely perceptible diurnal fluctuation, usually less than 0.02 foot, was noted when either of the irrigation wells $10/33-27L1$ and $10/33-27G1$. which are both about 800 feet distant, was pumping. In contrast, diurnal fluctuations of over 2 feet were observed in well $10/35-7G3$ when irrigation well $10/35-7F1$, which is over 1,400 feet distant, was pumping.

These marked differences in response of water levels to pumping in these two areas support the inference that the seasonal fluctuations in the intake area are due to the composite effect of recharge mounds which tend to give high levels in winter, and withdrawals by pumping augmented by natural depletion, which tend to produce low levels in summer. These fluctuations represent changes in the amount of water in storage. On the other hand, the fluctuations in the area of confinement represent pressure changes but essentially no unwatering of the deposits or changes in storage. In the area of confinement some small changes in storage actually do take place (Wenzel, 1942; p. 99), but they are so insignificant when compared to the changes in the mtake area that they are not considered in this report.

FLUCTUATIONS CAUSED BY A MOVING LOAD ON THE LAND SURFACE

Momentary rises and declines of water levels of 0.02 to 0,05 foot caused by passing trains have been observed in wells in the area of confinement in the lower Santa Ynez Valley during the course of the ground-water investigation in Santa Barbara County. Similarly, Stearns (Stearns, Robinson, and Taylor, 1930, pp. 148-150, figs. 20, 21) observed rises of between 0.01 and 0.03 foot in certain wells in Mokelumne area, California; and Jacob (1939, pp. 666-674) made an intensive study of this type of fluctuation on Long Island, New York.

In the Santa Maria Valley area fluctuations caused by passing trains were observed in well 11/35-33G1, which is in the area of confined water and 58 feet from the Southern Pacific railroad. The well penetrates the full thickness of confining material, constituting the upper member of the alluvium, and is reported to penetrate the main water body for a depth of over 30 feet. Figure 7 shows the fluctuations of water level before, during, and after each of two trains passed on $March 7, 1946.$ The general decline of water level in well $11/35-33G1$. during the period of observation is due to pumping from a nearby were observed in well $11/35-33G1$, which is in the area of confine water and 58 feet from the Southern Pacific railroad. The well penetrates the full thickness of confining material, constituting the upper member of the

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FIGURE 7. - Hydrograph of well 11/35-33(1, showing the effect of a moving load on the land surface

The first train to pass was a slow-moving freight, and the second, about 30 minutes later, was a fast-moving passenger train. The effect on the water level in the well, as determined by individual tape measurements, seems not to have been the same in each case, but might have been found to be essentially the same had measurements been spaced more closely. The fluctuations of water level caused by the passing of the freight train seem to fit closely the explanation given by Jacob (1939, pp. 672-673, fig. 6) for the fluctuations in certain wells on Long Island.

The water tapped by well 11/35-33G1 is confined, and the aquifer is assumed to be elastic. Under these conditions, and in accord with Jacob's explanation, the fluctuations can be explained as follows. As the train approached the vicinity of the well the extra load on the confining bed caused an increase of pressure in the aquifer, resulting in a rise of water level in the well. With the passing of the train the load remained about constant for a time, but as the aquifer was compressed under the extra load, water was driven laterally and most of the train's load ultimately was supported almost entirely by the aguifer. Hydrostatic pressure then returned toward normal, and the water level in the well approached normal level. As soon as the train had passed the vicinity of the well, the excess load decreased, the aquifer, being elastic, expanded; and for a time the hydrostatic pressure in the aquifer was negative, resulting in a decline of water level in the well below normal level. As the water returned with

the expansion of the aquifer to its original shape, the hydrostatic head approached normal and the water level in the well returned to its normal position.

THE RELATION OF NET CHANGES OF WATER LEVEL TO NET CHANGES IN STORAGE

In the foregoing paragraphs the types of water-level fluctuations have been discussed from the standpoint of their causes, such as recharge and pumpage, which produce rises or declines most readily apparent over short periods of a few days or months. Because these causes also operate intermittently or continuously, they produce a composite effect which may result in a net rise, net decline, or no change in water levels over any particular period. In the intake area these net changes represent net changes in the amount of water in storage in the main water body. For example, over a given period if the total recharge is greater than the total discharge the difference goes into storage in the basin, and water levels show a net rise; if recharge is less than discharge the difference is taken from storage and water levels decline; and if recharge and discharge are equal, water levels show no net change. Because there have been relatively long periods of both above-average and below-average recharge, these net changes have been most pronounced over long-term periods.

Figure 8 shows fluctuations of water levels in the two wells having the longest record in the area and their relation to rainfall at Santa Maria. The continuous record of fluctuations in well 10/34-14E3 began in 1917, and in well 9/32-7N2 in 1920. Prior to 1918 few recorded data are available, but enough reports and records were obtained from owners and well drillers in the course of the investigation to determine in a general way the major fluctuations that took. place in well 14E3 from 1903 to 1917. For example, the measurement for the year 1903 is based on a reported water level in a nearby well; and those for the years 1906 and 1907 are also based on levels in nearby wells given in Lippincott's report.¹⁶

The indicated decline of water level between 1890 and 1903 is based partly on reports of early water levels and partly on the remainder of the recorded fluctuations. Prior to 1883, according to Mason (1883, pp. 312-313), flowing water was obtained at a depth of 110 feet in and near Guadalupe, and in about 1880 water was obtained on the Rancho Punta de la Laguna (pl. 1) at depths of 20 to 60 feet. Thus, there must have been a net decline between 1880 and 1907 of at least 20 feet at Guadalupe and possibly of more than 30 feet in the Rancho Punta de la Laguna. (See pl. 6.) From these data, and from the general parallelism of the hydrograph with the curve for

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¹⁴ Lippincott, J. B., op. cit., diagram No. 3, 1931.

accumulated departme of rainfall, the hydrograph of 14E3 is extended back to an estimated depth to water of 50 feet in 1890. Not only is this partly reconstructed record the longest available, but also the well is near the middle of the Santa Maria plain and within the pumped area. Although the records show that fluctuations in wells near the central part of the Santa Maria plain have had a wider range in amplitude than wells either near the coast or in the Sisquoc valley, the fluctuations are probably fairly representative of fluctuations within the valley area as a whole.

Study of the fluctuations of water level in well 10/34-14E3 and comparison with the rainfall as a measure of the recharge reveal several pertinent features with regard to changes in storage. First, there have been two periods of rising water level, indicating increase in storage, and two periods of declining water level, indicating decrease in storage. Second, the long-period changes of water level, and hence storage, have been generally.proportional to the natural fluctuation of rainfall. Third, the water level in 1944, which probably is close to a long-term peak, was about 35 feet lower than the peak of 1918. Thus, during the period 1918-44, pumpage has apparently been sufficient to modify considerably the natural fluctuations of water level. In ensuing paragraphs the long-term fluctuations are discussed according to four main periods of water-level change; namely, the period 1890-1904, of declining water level; the period 1905-18, of rising water level; the period 1919-36, of declining water level; and the 'period 1937-44, of rising water level.

Net decline during the period 1890-1904.-The period 1890-1904 was one of below-average rainfall (fig. 8), and hence below-average recharge. During the first 8 years of the period essentially all groundwater discharge was by natural processes. Doubtless there was not only maximum discharge by ground-water outflow (p. 96), but also a considerable quantity of discharge by ground-water overflow. After 1898, pumpage began to extract limited quantities of ground water. This pumpage, together with deficient rainfall, evidently caused a decrease in storage during this period. The net decline of water level may have amounted to as much as 45 feet at well $10/34-14E3$. Had there been no pumping during the latter 7 years of the period, the decline would have been somewhat less, but the exact amount cannot be ascertained.

Net rise during the period 1905-18.-The above-average rainfall from 1905 to 1918, which is best illustrated by the graph of accumulated departure from average rainfall for that period (fig. 8), produced above-average quantities of runoff in the Santa Maria and Sisquoc Rivers_{rax} Consequently, storage increased during this period because recharge from all sources exceeded natural and artificial discharge.

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The increase caused a net rise in water levels, which amounted to about 40 feet in well 14E3. On December 22, 1918, the water level in well 14E3 stood only 58,67 feet below the land surface-the highest on record. It is reported that water levels in other wells also reached their highest stages in this year.

The area of flowing wells at the west end of the plain extended farthest eastward in 1918 (pl. 5). Increased pumpage and groundwater outflow together, however, apparently were insufficient to prevent the natural increase in ground-water storage and the accompanying rise of water levels. Consequently, along the eastern boundary of confinement the water level in 1918 stood only about 15 feet below the surface of the Santa Maria plam, and ground~water overflow into the streams occurred. The pits which had been constructed for the early pump installations (p: 84) were inundated by the rise of water levels, and the pumps had to be raised in those wells not already abandoned. For example, it is reported that in 1916 or 1917 a pump near well *14E3* was covered by the rising ground water, It was necessary to send a diver down to unbolt the submerged pump' and to raise it above the water level in the pit.

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Net decline during the period 1919-36.—The favorable period of increased storage which reached a peak in 1918 was followed by an 18year period in which water levels declined rapidly, and storage reached its historic low in 1936. The general area-wide conditions are best shown by the water-level contours for that year *(pl. 5)*. The water level in well 10/34-14E3 declined from the highest recorded level of 58,67 feet below land surface on December 22, 1918, to the lowest recorded level of 132.69 feet on October 18, 1936-a total net decline of 74.02 feet. In the Sisquoc valley the water level in well $9/32-7N2$ declined from 52.7 feet below the land surface on May 11, 1920, to the lowest of record of 99.7 feet in January 1935—a total decline of 47 feet. In those wells which have shorter records (figs. $3-6$) the water levels all showed similar declines in the latter part of lowest recorded level of 132.69 feet on October 18, 1936—a total decline of 74.02 feet. In the Sisquoc valley the water level in v $9/32-7N2$ declined from 52.7 feet below the land surface on May 1920, to the lowest of rec

The relatively rapid decline of water levels during the I8-year period can be directly attributed to two major causes. First, rainfall was considerably below average (fig. 8) and therefore recharge was. small (table 7). Second, the introduction of vegetable farming in the early twenties greatly increased the withdrawals from storage by pumping for irrigation (tables 8 and 9). Thus, from 1919 to 1936 natural discharge plus the increased artificial discharge were considerably in excess of the below-average recharge. Consequently, a steady depletion of storage occurred, accompanied by a lowering of water levels throughout the area. In well $10/34-14E_3$ the rate of \mathbb{R}^3 Plate 5 shots. decline averaged over 4 feet per year.

The economic effects of the decline in water levels were widespread. In 1918, when there was a relatively large area of artesian flow from wells, most wells at the west end of the valley were equipped with centrifugal pumps: As the area of flow contracted owing to the decrease in head, pumping levels fell below the physical reach of suction pipes, and deep-well turbine pumps had to be installed. In the intake area, where most wells were equipped with deep-well turbine . pumps, pumping levels locally fell below the bottom of suction pipesa condition which necessitated the lengthecing of most pump columns. A few wells were ultimately deepened to obtain a sufficient quantity of water. However, by far the greatest economic effect was the increased cost of pumping due to the area-wide increase in pumping lifts.

In the area of confinement water levels were depressed considerably by 1936, but there was always a favorable seaward gradient and thus a movement of ground water in that direction (pl. 6). The hydrograph of well $10/35-7F1$ (pl. 6) also shows that the water levels near the coast during this critical period were above sea level. Therefore, even during this period of lowest water levels, there was sufficient fresh-water head to prevent encroachment of sea water into the range of thickness penetrated by wells. However, there, theoretically was encroachment into the basin, at depth (p. *138).*

Net rise during the period 1937-44. - Following the 18-year period of below-average rainfall and the consequent depletion of storage, water levels throughout the area rose from the historic low of 1936 to relatively high elevations in 1944 in response to a period of above-average rainfall and recharge. The rise took place even though there was a steady increase of pumpage (tables 9 and 10), The water levef'in well $10/34-14E3$ rose from 132.69 feet below the land surface (lowest level of record) to 95.40 feet on March 12, $1944-$ a total net rise of ,37.29 feet. However, the water level 'in ,March 1944 w'as still 36.73 feet below that of December 22, 1918, and was about the same as the reported level in 1903. Similarly, the water level in well 9/32-7N2 in the Sisquoc valley showed a net rise of 49.5 feet, from 99.7 feet below the land surface in January 1935 to 50.2 feet at the end of 1940, slightly above the previous high level of 1920. The hydrographs of other wells (figs. $3-6$) whose records are considerably shorter than those of wells $9/32-7N2$ and $10/34-14E3$ show declines of water levels to about 1936 and a subsequent rise into 1944. In most of these wells the levels in the years 1941-44 are the highest of record simply because their records do not extend back far enough to indicate the early conditions.

In NoNework-5 shows water-level contours for the main water body during the period of low water levels in 1936 and in the spring of 1942. The

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net rise of water levels is clearly indicated by the comparison of contour lines in these two years. Furthermore, the plate shows that the area cif flowing wells increased substantially from about 1 square mile in 1936 to more than 5 square miles in 1944 but was considerably less than the maximum area of flow in 1918. .

Thus, the net rise in water levels during the period 1937-44 indicates an over-all net increase in storage in the intake area during this period. Storage beneath the Santa Maria plain may have been about equal to that of 1903 or 1907 (pl. 6) but was considerably below that of 1918; and in the Sisquoc valley, where pumpage is·small, the water levels indicate a net increase in storage about equal to the net. depletion in the years 1920-36.

The stage of Guadalupe Lake apparently has varied considerably . during the past 3 decades. It is reported that in 1918 the lake surface was relatively high, and that thereafter the level fell progressively until the lake went completely dry in 1934-at no time before had the lake ever been known to be dry. From 1934 into 1937 the lake bottom was farmed, but in 1938, with the rise of water levels, the lake was reestablished. In 1942 it was observed that the elevation of the lake surface corresponded roughly to the elevation of water levels in levels in wells $10/35-26K1-10$, situated on the lake shore, thus indicating a hydraulic continuity with the main water body. Examination of old shore lines showed that the lake at some time had been about 5 to 10 feet above the level of 1942. Thus the reported stage of Guadalupe Lake has corresponded in general to the major fluctuations of ground-water levels throughout the area.

Significance of long-term net changes.-Thus, it is believed that under natural conditions there has been a fairly delicate balance between. recharge and natural discharge. The' large fluctuations of water levels in the intake area, in the early years before any appreciable pump age, indicate this relationship. During years of high natural recharge there was an increase in ground-water storage, and during years of low recharge, storage decreased. Pumpage in years prior to 1920 probably was not large enough to affect appreciably the amount of water in storage at any time. However, after 1920 the rapid in~· crease in pumpage affected storage considerably. Ooupled with .and augmented by deficient rainfall, the increased discharge caused a progressive and large decrease of storage into 1936 .. In the period 1936-44, one of above average rainfall, although discharge was not great enough to exceed the recharge, it was great enough to prevent the restoration of water levels to the peaks reached in 1918. .

Thus, at least by 1936, and probably earlier, the dynanric balance established between natural recharge and total discharge owns such nd stawww.MarkawWipTaxycom that water levels ever since have fluctuated at levels considerably

below those that would have prevailed under natural conditions. Consequently, it is believed that water levels will continue to fluctuate in accord with protracted wet and dry periods, but that the amplitude of the fluctuations will be greatly modified by the pumpage at that time. With an expected increase in pumpage, during protracted periods of below-average recharge, water levels may-decline to or . even below the levels of 1936; and during periods of above-average recharge they will undoubtedly rise, but probably never again will they reach peaks such as the levels attained in 1918.

Thus, the long-term net rises or declines of water levels within the intake area indicate net increases or decreases of ground-water storage, respectively. Because the amount of water-level' change is directly proportional to the corresponding change in storage, the actual amounts of storage change can be determined when the specific yield of .the water-bearing deposits within the zone of water-table fluctuations is known. This concept is developed in the following pages,

NET CHANGES IN STORAGE IN THE MAIN WATER BODY:

METHODS FOR ESTIMATING STORAGE CHANGES

It has been shown in the preceding section that over periods of years net changes in water levels accompany net changes in groundwater storage in the intake area. These changes in storage are converted to actual quantitative estimates by two methods. During the years 1929-45, the quantities can be estimated for any period simply by taking the difference between total recharge and discharge for the period. However, because these totals themselves are in part estimated, it is desirable to derive estimates of net change by another method in order to verify the totals. The other method employed is the use of the specific yield of the deposits within the zone of watertable fluctuations, applied to the net change in water levels.

USE OF SPEOIFIO YIELD

. ESTIMATE OF SPECIFIC YIELD

The specific yield of a rock or soil with respect to water is usually expressed as a percentage derived by dividing (1) the volume of water which a rock or soil, after being saturated, will yield by gravity by (2) the volume of the rock or soil (Meinzer, 1923, p. 28). In the field, the specific yield is derived by dividing' the increase or decrease in stored water in a given area by the average rise or decline of the water table in the same area.

The method used for estimating the specific yield of the waterbearing deposits in the zone of water~table fluctuations in the Santa wwwMarkawWhilesycourea is patterned after that used by Piper (Piper, Robinson, and Park, 1939, pp. 74-76) in the Harney Basin, Oregon,

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and by Eckis (1934, p. 109, table 5) in southern California. It is based on the relative volumes of gravel, sand, and clay that lie within the zone of water-table fluctuations, taken in conjunction with adjusted values of specific yield for each type of material as determined in other areas. This method was selected primarily because of the wide lithologic variations that exist between the different waterbearing formations and within each formation itself, Ultimately, if desired, the estimates could be refined by extensive pumping and laboratory methods which, however, are beyond the scope of this, investigation.

The formations underlying the Santa Maria Valley area and in which the water-level fluctuations have occurred primarily are: the coarse-grained lower member and the lower part of the upper member of the alluvium beneath the Sisquoc plain and the intake area of the Santa Maria plain; the Orcutt and Paso Robles formations and locally the Careaga sand beneath the upland areas, and the Telatively coarse grained tenace deposits along the north side of the Sisquoc valley, For these deposits there are seven terms commonly used by well drillers to designate the various lithologic types of material encountered in well-drilling operations. These are: gravel, sand, silt, clay, gravel and sand, gravel and clay, and sand and clay. Well drillers questioned during the investigation all maintained that such terms as "gravel and sand" mean about half sand and half gravel; and the term is therefore evaluated accordingly. However, material described as "gravel and clay" and "sand and clay" are both considered as "clay" because it is believed that the pore spaces between the sand grains or pebbles are largely filled by clay, and hence the specific yield of these two types of material would approach that of clay. Because the term "silt" is commonly used to designate very fine sand and clay, material thus designated is also classed as clay. Therefore, the seven types of material as distinguished by well drillers are in this report divided into three main classes: gravel, sand, and clay.

Naturally, there is a considerable range in the specific yield of the gravel, sand, and clay, depending on grain size, degree of sorting, and . the terminology of the individual driller. Nevertheless, owing to the large number of well logs which were analyzed it is believed that a mean value for specific yield can be applied satisfactorily. The values used for specific yield are: for gravel, 30 percent; for sand, 20 percent; and for clay, 5 percent. These values are slightly lower for gravel and higher for clay than those used in the Harney Basin, because the term "gravel" as used by drillers usually contains some sand; and the term "clay," some sand and silt.

To obtain the relative volume of the three types obppatest hen 250 well logs were carefully examined, and for each log the footage, of gravel, sand, and clay in the zone of water-table fluctuations was determined. Owing to the wide range in footage of each type of material from one part of the area to another and furthermore because the footage of each type varied, in general, according to the areal distribution of the water-bearing formations, it was found advisable to divide the' area into three subareas, each containing generally the same types of material. The three subareas are as follows: the Sisquoc plain and the terrace to the north and that part of the Santa Maria plain from 0 to 10 miles west'of Fugler Point; that part of Santa Maria plain from 10 to 13 miles west of Fugler Point, or to the edge of the artesian area; and the Orcutt upland, the southwestern part of the Nipomo upland, and the dissected upland area south of the Sisquoc plain. Within each subarea the logs showed approximately the same percentages of gravel, sand, and clay. The percentages thus derived are believed to be representative of the total quantities' of the three principal classes of material distinguished in the zone of water-table fluctuations in each subarea.

The following table shows the extent of each subarea, the number of well logs, the percentage volume of each class of material, and the calculated specific yield of the material in the zone' of water-table fluctuations. For each subarea the figure for specific yield of the material is the sum of the products of the specific yield of gravel, sand, and clay times, the percentage volume of each. The average specific yield for the whole area is weighted in proportion to the relative areal extent of each subarea.

Estimates of the specific yield of water-bearing materials within the zone of water-, , *table ftuctuahons* m *the Santa Maria Yalley area*

Subarea	Агеа (acres)	Num- ber of well logs	Percentage volume			Specific yield
			Gravel	Sand	Cla v	(per- cent)
1. Sisquoe plain, terrace to the north, and part of Santa						
Maria plain 0 to 10 miles west of Fugler Point	21,900	136	44	39	17	21.8
2. Part of Santa Maria plain 10 to 13 miles west of Fugler Point.	19,600	65	23	41	36	15.3
3. Orcutt, Nipomo, and minor up- land areas____________	50,900	80	30	21	49	15.6
Total for area ₋₋₋₋	82, 400	281.	Weighted average			17.2

The specific yield differs considerably from one subarea to another. It is high when the percentage of gravel is high and that of clay is found www.NoNewWipTes wersa. In general, the weighted average specific yield

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for the area is relatively high because, in all subareas, the percentage volume of clay is less than 50 percent of the total.

APPLICATION TO WATER-LEVEL CHANGES TO OBTAIN STORAGE CHANGES

If water levels change uniformly throughout the intake area, the net changes in storage could be determined simply by multiplying the specific vield times the area in acres times the net change of water level in feet. For example, a net rise of 1 foot throughout the 80,000 acres of the intake area would represent an increase in storage of about 14.000 acre-feet. However, it has been shown that the water table does not rise or decline uniformly. Hence, the figure for specific vield of 17.2 percent for the total area is not strictly applicable. Consequently, changes in storage have to be computed separately for each of the three subareas, then totaled.

In order to obtain the net change in water level, contour maps spanning desired periods were drawn from peak water levels in the spring months of the 2 years being compared. One was then superimposed over the other, a grid of half-inch squares was laid over the two, and the net change in water level determined in each square of the grid. The figures in the squares in each subarea were averaged separately to obtain the average net change of water level for each. Thus, the average net change in storage in acre-feet for the intake area. over any desired period of time for which sufficient water-level data. are available can be obtained by adding the products for each subarea of: the average net change of water level in feet, the area in acres, and the specific yield.

The accuracy of the results obtained by the use of this method is dependent not only on the validity of the figures for specific vield, but also on the detail of the contour maps. Owing to the irregularities and ever-changing shape of the water table, due primarily to pumpage and recharge, numerous nearly simultaneous measurements in wells are necessary to obtain an accurate, detailed contour map. Even now there are too few wells in which measurements can be made around the margins of the main water body to control the contours accurately (pl. 5). In the past, and for a particular time desired, even fewer measurements were available. Consequently, the computations of storage change by use of the specific yield method are somewhat in error, probably largely owing to this cause.

USE OF RECHARGE AND DISCHARGE

The second method by which net changes in ground-water storage can be estimated is by use of the estimates of total recharge and discharge which have been computed for the years $1929 - 45$ (tables 7) and 12). The difference between recharge and discharge over any

desired period of time within the years 1929-45, then, will give the net change in storage. However, estimates of storage change derived by this method are subject to considerable error because a small percentage error in estimating recharge will result in a very large percentage error in the amount of storage change computed.

For simplicity of treatment in the following sections, the recharge for any one water year, such as 1930-31, is listed as recharge in the second year indicated, in this case 1931. This is because the second year includes most of the water year, and because most of the recharge occurs after the beginning of the second year.

ESTIMATES OF STORAGE CHANGES, 1929-45

Estimates of net change in ground-water storage by use of specific yield and recharge and discharge have been made for three periods: for 1929-36, to show the net decrease in storage during the latter part of the dry period which began in 1919; for 1936-45, to show the net increase in storage in the current wet period; and for 1929-45, which spans the entire period for which estimates of recharge and discharge have been made.

In order to compare the net changes in storage determined by use of the two methods, both strictly should span identical periods. Theydo not, but the difference between them is relatively small. In the computations based on specific yield, contour maps were drawn for the spring peaks of 1929, 1936, and 1945, and net changes in storage for the three periods were computed between the spring peaks of the first and last years of each period (p. 119). On the other hand, the use of the estimates of total recharge and discharge is limited to water and calendar years, respectively, and net changes in storage for the three periods are computed by the differences of total recharge and discharge over an equal number of water years (ending September 30) and calendar years, respectively.

Figure 9 shows the time intervals that correspond to the three elements, net-change year, recharge year, and discharge year; and their chronologic relationship to each other for the period 1929-36. As shown, the time intervals are not exactly coincident, and hence the total net changes in storage computed are not strictly comparable. For example, the net water-level changes are taken from the spring peaks of 1929 to those of 1936, and the change in storage computed from these by the specific yield method are for that period. The net change in storage computed from total recharge and discharge is actually for the over-all period January 1, 1929, through September 30, 1936, and utilizes the difference between total discharge in the calendar years 1929-35 and total recharge in the water years 1930-36. W.The Way When the period 1929-36 determined

by use of total recharge and discharge incorporates the small amount of discharge in 1929 prior to the spring peaks, and that part of the recharge in 1936 after the peaks had passed, neither of which is included in the net change for the period as computed by use of specific vield.

Accordingly, not only does the computation by recharge and discharge differ somewhat within itself in regard to time interval, but it also differs slightly from that spanned in the specific yield method. Therefore, in estimates for periods of only a year or two considerable error may be introduced; but for longer periods, such as those considered in table 13, the error is reduced to a minimum and is probably. well within the limits of the errors involved in the estimates themselves.

The same principles apply to the remaining two periods. The net change in storage for the period 1936-45 computed by use of specific vield is best compared with the difference between total recharge for the water years 1937-45 and total discharge for the calendar years 1936-44; and similarly, for the period 1929-45, it is best compared with the difference between total recharge for the water years $1930-45$. and total discharge for the calendar years 1929-44.

Table 13 shows estimates of net change in ground-water storage for the three periods 1929-36, 1936-45, and 1929-45 as determined by use of the specific yield method and by the use of the totals for recharge and discharge; it also shows the difference between the results obtained by the two methods. Copy of document found TABLE 13.—Estimates of recharge, discharge, and net change in storage in the main water body during the periods 1929–36, 1936–45, and 1929–45

The table shows that the results obtained by the two methods differ by 2 to 25 percent of the total quantities involved in each period. However, discrepancies between these results are believed to be reasonable, considering the available data. Accordingly, the quantities obtained by the two methods are sufficiently in agreement not only to verify the general order of magnitude of values derived, but also to substantiate the methods used.

PERENNIAL YIELD OF THE WATER-BEARING DEPOSITS

The perennial yield of the water-bearing deposits in a coastal area is the rate at which water can be pumped from wells year after year without decreasing the storage to the point where the rate becomes economically infeasible, the rate becomes physically impossible to maintain, or the rate causes the landward migration of sea water into the deposits and thus renders the water chemically unfit for use. In the Santa Maria Valley area only the first condition was approached and that only locally during the mid-thirties, when pumping lifts were relatively high. Fortunately there has been at all times an appreciable seaward hydraulic gradient at the coast (pl. 6), and thus the danger of landward migration of sea water has never become serious. Similarly, the second condition has not been approached because the water body is so thick that under conditions of excessive pumpage and low recharge the first or third condition would be realized ww.Nonewwyprax.com
The yearly pumpage in recent years has been $930370 - 51 - 9$

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large enough to exceed the perennial yield, and with an expected increase in ground-water development, and especially during a series of dry years, pumpage may far exceed the perennial yield in the future.

In terms of water available for pumpage, the perennial yield may be expressed in a different way... Originally, under natural conditions and without any pumping the long-term recharge necessarily was equal to long-term discharge. However, for shorter periods of time recharge was greater or less than discharge, depending directly on climatic conditions; and the differences, as discussed on pages 111-117, caused either an increase or decrease in storage which was reflected in a net rise or decline of water levels throughout the area (fig. 8). Even with the subsequent development of pumpage, storage changes continued to be governed largely by climatic conditions, and hence, by recharge. Thus, it follows that during periods of large recharge more water can be pumped without decreasing storage, and during periods of low recharge less water can be pumped without decreasing storage.

Specifically, for any of these short periods the short-term yield is the total recharge less the total natural discharge plus whatever water there is in storage above the limiting factors for safe withdrawal. However, if little or no water is available in storage above the limiting amount for the period, the short-term yield is merely the difference between average yearly recharge and average yearly natural discharge. The long-term or perennial yield, on the other hand, is intermediate between the short-term yields of periods of above-average and belowaverage recharge, but it is not dependent on the available water in storage, whose fluctuations affect only the short-term yields. For all practical purposes perennial yield is the difference between longterm average yearly recharge and the average yearly natural discharge.

In the ensuing pages, these principles are applied to the Santa Maria Valley area, and quantitative estimates of short-term yield are made for the period of below-average recharge 1929-36, and for the period of above-average recharge, 1936-45. Also, an estimate of the perennial yield is made. The estimates are based principally on the estimates previously derived for recharge, discharge, and storage changes. (See tables 7, 12, and 13.) As discussed elsewhere (pp. 121-123), the estimates for recharge, discharge, and storage changes are for periods that are slightly out of phase with each other, and they are treated accordingly.

SHORT-TERM YIELD DURING THE PERIOD 1929-36

The period 1929-36 was one of below-average recharge, when total discharge exceeded total recharge, and it marked the end of a long period of storage depletion that began in 1919. Although there was a progressive depletion of storage during this period, the short-term

vield as defined was not actually exceeded because there had been a large amount of water in storage at the beginning. However, the short-term vield was approximately reached near the end of the period. when storage had been depleted to the point where pumping for irrigation locally exceeded economic limits. At that time the yield was approximately the difference between yearly recharge and the natural discharge by ground-water outflow to the sea. It probably would have been exceeded, all other factors remaining constant, had the pumping rate been continued long after 1936, and had recharge remained low.

These conditions can be expressed quantitatively as follows: The average yearly recharge for the period was about 34,000 acre-feet (p. 83), and the ground-water outflow in 1936 was about 9.500 acrefeet (table 11). Thus, at the end of the period the short-term yield was the difference between the two, or about 25,000 acre-feet a year. However, during the period the total net pumpage for irrigation plus the total pumpage for other use amounted to about 317,000 acre-feet (tables 9 and 10), or averaged about 45,000 acre-feet a year. This rate of pumpage was within the short-term yield because there was considerable excess water in storage that was being drawn upon. However, if these conditions had been maintained beyond 1936 the vield would have been exceeded by about 20,000 acre-feet per year.

The total net depletion in storage during the period 1929-36 amounted to between 159,000 and 200,000 acre-feet (table 13), or averaged about 26,000 acre-feet per year. The relatively uniform rate of decline of the water level in well 10/34-14E3 (fig. 5) throughout the 18-year period 1919-36 of below-average rainfall suggests that the rate of decrease in stored water was about constant. Accordingly, the rate of storage depletion of about 26,000 acre-feet a year for the period 1929-36 may be applicable uniformly to the entire period. If so, the total depletion of storage from 1919 into 1936 must have been roughly 500,000 acre-feet.

Obviously, had the average yearly recharge remained only 34,000 acre-feet, then the yield of the deposits would have continued to be exceeded beyond 1936, water levels would have continued to decline. and ground-water outflow to decrease. Ultimately, under such conditions pumping lifts would have been extremely high and perhaps in most of the area economically infeasible; landward encroachment of sea water into the water-bearing deposits eventually would have occurred. Furthermore, a large part of the highly productive lower member of the alluvium would have been unwatered, and wells would have been drawing from the less permeable Paso Robles formationa gondition which probably would have increased considerably the pumping lifts and hence the operational costs.

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> Quite possibly the average yearly recharge of 34,000 acre-feet is representative of periods of below-average rainfall. *If* so, the shortterm yield of about 25,000 acre-feet a year is on the order of magnitude to be expected near the end of future similar periods of below-average recharge. With present withdrawals approaching 65,000 acre-feet a year and total discharge approaching 80,000 acre-feet a year, it is obvious that during future dry periods storage, will be depleted at a, rate greater than that which took place in the years 1929-36.

SHORT-TERM YIELD DURING THE PERIOD 1936-45

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Fortunately, the periods of below-average recharge have been com~ :pensated for by complementary periods of above-average recharge, such as that for 1936-45. ' Total recharge during this period was far' greater than total discharge. Consequently, there was a considerable net increase in storage and the short-term yield for the period was never approached.

These conditions are expressed quantitatively in much the same manner as for the period 1929-36, as follows: The average yearly recharge for the period was about 98,000 acre-feet (p. 83), and the, average yearly ground-water outflow was about 11,000 acre-feet '(table 11). Thus, the short-term yield was the difference between the two, or about 87,000 acre-feet a year. During the period the total net pumpage for irrigation plus the total pumpage for other use amounted to about 521,000 acre-feet (tables 9 and 10), or averaged about 58,000 acre-feet a year. Thus, pumpage averaged about 29,000 acre-feet a year less than the short-term yield.

acre-feet a year less than the short-term yield.
The total net increase in storage for the period 1936-45 amounted to between 260 000 and 264,000 acre-feet (table 13), or averaged about 29,000 acre-feet a year. Thus, in contrast to the preceding period, storage increased considerably, and water levels rose accordingly throughout the area (figs $3-6$). In fact, the storage was enabled to regain about one-half of the estimated over-all depletion of about;
500,000 acre-feet incurred during the period 1919-36. where two, or about 87,000 acre-feet a year. During the period the total and the two, or about 521,000 acre-feet (tables 9 and 10), or averaged about 58,000 acre-feet a year. Thus, pumpage averaged about 29,000 acre-feet a

The unusually large recharge in 1941, which was about 230,000 acre-feet, was nearly double the quantity supplied in most wet years $(table 7)$. Consequently, the short-term yield of about 87,000 acrefeet a year for the relatively short period 1936-45 is probably greater than that for other longer wet periods such as $1905-18$ (see fig. 8), and probably is above the general average that might be expected in future wet periods. The average yearly recharge for the years $1937-$ 45, exclusive of that for 1941, was about 82,000 acre-feet a year. Thus, by subtracting the average yearly ground-water outflow; of, $11,000$ acre-feet a year, it is believed that the short-term yield for result www.NoNewWip Pax.com average wet periods would be on the order of 70,000 acre-feet a year.

ESTIMATE OF PERENNIAL YIELD UNDER NATURAL CONDITIONS

Obviously, the perennial or long-term yield of the water-bearing' deposits in the Santa Maria Valley area is a quantity greater than the short-term vields during periods of below-average recharge, but less than, the short-term yields for periods of above-average recharge. To , obtain the' maximum perennial yield, it is desirable to reduce to a, minimum the natural discharge by ground-water outflow, but not to the point where either the water levels are below practical limits, or the danger of salt-water encroachment becomes imminent. On the other hand, it is undesirable to permit storage to increase to the point where losses by ground-water overflow and evapotranspiration occur as they did around 1918. In addition, it is desirable to stop theloss by flow from wells.

The perennial yield is estimated by equating certain of the quantities derived in preceding sections of this report based on the somewhat above-average period 1929-45. The estimates obtained are then modified on the basis of rainfall to the long term. Two independent methods' are used commonly for estimating perennial yield which can be applied to the Santa Maria Valley area for this period, as follows: Perennial yield is equal to the total recharge (table 7) less the total natural discharge by ground-water outflow $(p, 96)$ divided by the number of years of inventory; and it is equal to the total net pumping draft (tables 9 and 10) plus the net increase in storage (determined by specific yield method, table 13) divided by the number of years of inventory. These may be expressed in equations, respectively, as follows:

$$
Perennial yield = \frac{1,121,500 - 180,000}{16} = 58,800
$$
 (1)

$$
Perennial yield = \frac{743,000 + 95,200 + 60,000}{16} = 56,100
$$
 (2)

Because these two quantities agree very closely, the perennial yield, based on the relatively short period 1929-45, is considered to be the, average of the two, or is estimated to be about $57,000$ acre-feet a 'year.

However, because rainfall during the period 1930-45 compared to that of the long term is above average, this estimate of perennial yield is modified accordingly. The basis for the modification rests solely upon the rough correlation that exists between rainfall and recharge. and hence perennial yield. At Santa Maria the average rainfall for the 16 -year period $1930-45$ was 15.42 inches, whereas for the 60-year

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period 1886-1945 it was 14.40 inches (table 1); that is, the long-term average rainfall was about 93 percent of that for the shorter period. Although it is almost a certainty that the perennial yield during the 60-year period was not also exactly 93 percent of that for the shorter period, nevertheless, in the absence of other data this percentage is used. The estimated perennial vield of about 57,000 acre-feet a year for the period 1929-45, then, is adjusted to the long term by multiplying by 93 percent. Thus, under natural conditions the perennial yield of the water-bearing deposits in the Santa Maria Valley area is estimated to be about 53,000 acre-feet a year.

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The validity of this figure is dependent on two critical factors. First, the adjusted value applies specifically to the 60-year period, $1886-1945$; and for it to apply in the future it must be assumed that the future climatic cycles, or periods of above-average and belowayerage rainfall and, hence, recharge, will continue to operate in the. same manner and will be on the same order of magnitude as they havein the past. With regard to the expectable future recharge, there eventually may be some reduction due to the development of injection with ground water in the Cuyama Valley (pl. 4). Withdrawals, which started in 1939, have increased to about 17,000 acre-feet in 1946, and probably will continue to increase. Because in the Cuyama' Valley recharge to ground-water bodies by seepage loss from the Cuyama River is limited by the rate of vertical downward percolation to the deep-lying water table, it is' believed that the relatively small amount of runoff which ordinarily passes the area of withdrawals will not be reduced appreciably. However, it is believed that increased pumpage may reduce substantially the natural ground-water discharge into the Cuyama River below 'the area of withdrawals. The amount so discharged at present is estimated to be about 5 second-feet, If it is assumed that because of evapotranspiration losses and diversions only 50 percent of this flow reaches the Santa Maria Valley area, and further that increased pumpage in the Ouyama Valley will eventually cause the flow to stop-entirely, then the future. recharge, and hence the perennial yield of the Santa Maria Valley area would be reduced by about 2,000 acre-feet a year. A more accurate estimate of this loss can be made only when more low-flow measurements are available below the area of ground-water discharge.

Second, the yield derived was based on a minimum practicable average ground-water outflow of 11,000 acre-feet a year. To salvage much more outflow would necessitate a lowering of water levels and, hence, costly pumping lifts. It is believed that the range of water levels between 1929 and 1945 is probably the most efficient from the, $\operatorname{standard}$ pumping of both maximum salvage and nominal pumping lifts.

 W ith respect to the current net pumpage of about 65,000 acre-feet a year (tables 9 and 10), it is apparent that the perennial yield is being exceeded by about 12,000 acre-feet, which in years of average recharge would result in a yearly average net decline of water levels' amounting to slightly less than 1 foot (p. 120). However, because there was in 1945 about 260;000 acre-feet in storage above the esti mated minimum levels, the yearly depletion of 12,000 acre-feet can be tolerated for a limited time. Obviously, if this yearly overdraft is continued or should be increased with no additional supply of water, the result will be a depletion of storage and a decline of water levels to stages far below those of 1936.

Two measures should be considered immediately to conserve ground water; Reduce or stop entirely the flow· from wells when not in use 'because with adequate' control the current discharge could be reduced by over 1,000 acre-feet a year (p. 91); and adopt more efficient methods of irrigation. For example, decrease to a minimum the unduly large quantities of "tail waste," space irrigation periods at the maximum practicable intervals, and eliminate wherever possible the long openditch conveyance of irrigation water from pumps to the land being irrigated. If these measures were adopted it might be possible to keep the pumpage within the limits of the perennial yield, or at about $53,000$ acre-feet per year and, at the same time, maintain the present agricultural development.

ESTIMATE OF INCREASED PERENNIAL YIELD BY SALVAGE OF SURFACE-, WATER OUTFLOW

A program for a more efficient utilization of the surface-water' resources of the Santa Maria River drainage system has been largely outlined by the Bureau of Reclamation, United States Department of the Interior.¹⁷ In general, this program involves the construction of two dams for surface-water storage or detention. One of the sites is on the Cuyama River about 5 miles above the mouth-a location known locally as the Vaquero Dam site. In conjunction with this structure, a silt-debris dam is to be constructed upstream. The other dam contemplated is on the Sisquoc River about 10 miles above its confluence with the Cuyama River, at a location known locally as. the Round Corral Dam site.

One of the fundamental purposes of this program is to salvage each year a substantial part of the surface-water outflow which is wasted to the sea, and so to' provide a means of increasing ground-water recharge. During the 16-year period 1930-45 this loss was estimated

¹⁷ Water requirement, water supply and flood control, Santa Maria Basin comprehensive plan, Santa BBabbithent found Sarbara County-project, Oalifornia; U. S. Bur. Reclamation Rept., appendix 8, 67' pp., March, 1946.
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to total about 566,000 acre-feet, or to average about 35,000 acre-feet a year--about 40 percent of the total runoff (table 5). The reason for the waste is that most runoff occurs during storms and at rates, too large for complete absorption through the channel deposits and, hence, transference to the main water body. Thus, as in many ground-water basins under natural conditions, the available surface-water resources have not been utilized fully in replenishing ground-water supply. However, if the winter flow can be largely-detained and released over a longer period of time; much of the waste could be salvaged as additional recharge. Thus, by increasing the recharge, and hence the perennial yield of the basin, the program would be of considerable importance to future ground-water development in the area.

Under the program, two features are critical with respect to the amount of increase of perennial yield from salvage of surface~water outflow, as follows:

1. Because water levels indicate' that ,at present there is ample room between the land surface, and the water table beneath the Sisquoc and Santa Maria River channels and adjacent areas to accommodate a large increase in storage, the rate at which water could be transfened from surface reservoirs to the main water body would be dependent solely upon the absorptive capacity of the channel deposits, Existing data show that the channels are dry or nearly so throughout more than half of each year, are quite permeable (table 2), and therefore provide excellent natural spreading grounds for the trans-,' ference of water stored in reservoirs.

2. The perennial yield could be increased by about the amount of surface-water outflow salvaged by the reservoirs, less evapotranspiration losses incurred from spreading operations and from the reservoirs themselves, and less any loss incurred from development in the Cuyama Valley (p. 128).

Under one plan presented by the Bureau of Reclamation,¹⁸ roughly 50 percent of the average yearly surface-water outflow could be, salvaged. Assuming the long-term average yearly outflow to be 93percent of the 35,000 acre-feet a year estimated for the period 1930-45 $(p. 128)$, it would amount to 33,000 acre-feet a year. If the perennial yield were to be increased by about one-half that amount, or by about 16,000 acre-feet a year (neglecting evapotranspiration losses), the estimated perennial yield of 53,000 acre-feet a year under natural conditions would be increased to about $65,000$ to $70,000$ acre-feet under this particular plan.

The net pumpage in 1944 was about $65,000$ acre-feet a year. Even if the ground-water conservation measures suggested on page 129 were to be adopted, the current draft would approach the perennial yield as increased under this particular plan of the Bureau of Reclamation. Therefore, this plan would correct the present deficiency, but it would not provide much if any margin for future development. On the other hand, the steady increase in pumpage during the 4 years 1941 to 1944 strongly suggests that further development will occur.¹⁹ A sustained rate of pumpage materially greater than the 1944 rate would have to be supplied by an increase in salvage of surface waters because, nearby'sources of water for importation are not available. Thus, it would appear desirable to plan now to salvage the largest amount of surface-water outflow that is economically practicable, and 'so to increase the yield accordingly; also, to limit ground-water development so as not to exceed the increased yield. Such a program, if accomplished, not only would provide for the maximum utilization of the water resources of the Santa Maria Valley area, but also would prevent a serious overdevelopment that would be detrimental to the $e^{\frac{1}{2}}$ economy of the entire valley area.

CHEMICAL QUALITY OF WATER GENERAL' FEATURES

. In 1941 and 1942, in connection with the field canvass of wells, the Geological Survey collected 152 samples of water from 116' wells for chemical analysis. Of these, 7 were analyzed for all constituents and the remainder were analyzed only for chloride and hardness in parts per million and for specific electrical conductance in reciprocal ohms \times 10⁵ (K \times 10⁵ at 77° F.), which is a measure of the total dissolved solids. In addition, two complete analyses were obtained of river water during low-flow conditions-one from the Cuyama River at its mouth and one from the Sisquoc River above its confluence with La Brea Creek. Numerous other agencies have made available 101' study over 350 analyses, mostly from water wells but in part from streams, lakes, and ponds. Records of representative partial analyses are included in table 14; and records of selected "complete" analyses are shown in table 15. The locations of all wells are shown on plate 1 and of all streams outside the area on plate 4.

1º Since the completion of this report, estimated net pumpage for the 5 years 1945 to 1949 has been 75,000, , :85,QOO, 100,000, gO,OOO,and 100,000 acre-feet, respectively. '

¹⁸ U. S. Bursau of Reclamation, op. cit., tables 5A and 5P, 1946.

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[Analyses by A. A. Garrett, Geological Survey]

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TABLE 14.—Selected partial chemical analyses of well waters in the Santa Maria Valley area—Continued

[Analyses by A. A. Garrett, Geological Survey]

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TABLE *IS.-Selected chemical analyses of well and stream waie,'s of the Santa lda'ria Valley area*

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The analyses show a considerable range in the chemical quality of the main water body from one part of the area to another. However, . the range in quality appears to bear little relation to range in depth of the wells—a fact which indicates that the water throughout the tapped limits of the main water body is at liberty to mix freely. It is believed, therefore, that the range in quality is due primarily to differences in the sources of water and to its subsequent alteration as it circulates underground and mingles with water from other sources. Accordingly, the quality of water in general is briefiy discussed as it appears in the Sisquoc valley where the principal source is the Sisquoc River, in the Santa Maria plain where the principal source is the Santa Maria River, and in the Orcutt and Nipomo uplands were the source is rain. In addition, the change in quality from place to place is also discussed. Ohloride contents of waters from wells near the coast are examined with specific references to the fresh water-salt water contact.

In the Sisquoc valley, the total solids content of the ground water is somewhat less than that of the upper range of concentration of the water in the Sisquoc River, based on three analyses of river water sampled in 1942 and in 1943. In the three samples analyzed, the total solids content ranged from 420 to 770 parts; that of the ground waters adjacent to the river ranged about from 610 to 640 parts, computed from electrical conductivity ... The river and ground waters range in chloride content from 9 to 23 parts and 23 to 28 parts, respectively. The ground water is definitely higher in hardness than the river water, ranging from 400 to 750 parts.

Well 10/33-35R1, also in the Sisquoc valley, yields water similar to the water in the Cuyama River. Both these are calcium, sodium sulfate waters in which the total solids contents are over 1,100 parts per million. Of the two, the river water is somewhat more concentrated. The similarity indicates that waters percolating from the Cuyama River, extend southward beneath the Sisquoc River at least to well 35R1, where they are only slightly diluted by the less concentrated waters of the Sisquoc.·

In the Santa Maria Valley, the quality of ground-water is sitnilar to that in the Santa Maria River. 'During periods of flow the quality of the water in the river is necessarily a blend of the qualities of the water in the Cuyama and Sisquoc Rivers, deperiding on the quantity of each. Hence, the quality of water in wells varies accordingly. The Cuyama River in its upper COUTse traverses formations which contain large amounts of gypsum, hence the water would be expected to be high in total solids, owing to solution of calcium and sulfate. The analysis of water from the Cuyama River above Alamo Oreek (table 15) is

confirmatory and represents essentially a calcium sulfate water, in which calcium and sulfate contents are 520 parts and 1,800 parts, respectively. The water in wells along the'Santa Maria River has a chloride content ranging between 30 and 60 parts per million, a hardness between 500 and 700 parts, and total solids between 1,000 and 1)600 parts. '

.. [~]

Southward across the Santa Maria plain the chloride content, hardness, and total solids increase somewhat. However, there is a relatively rapid decrease in the concentration of all three southward beneath the Orcutt upland. Toward the coast the quality improves, except in a local area along the south edge of the plain in T , 10 N., R. 35'W., and extending up 'along the creek southeast to Guadalupe Lake, nearly to the town of Orcutt. In this area the chloride content is over 100 parts per million, and in one well it reaches the maximum in the area of 175 parts. The cause of this increase is not definitely known, but it may be due in part to seepage of contaminated water from surface sumps or waste ponds. Because the base of the waterbearing deposits lies at least 1,000 feet below the bottoms of these wells, the higher chloride content is not believed to originate from below. However, it may be said that the condition has not changeQ. materially since 1927 in those wells for which data are available.

Beneath the Orcutt upland and particularly in the vicinity of the city of Santa Maria wells (9/34-3N1-3), the waters range from 46 to 94 parts per million in ohloride content, from 90 to 130 parts in hardness)' and from 200 to 320 parts in total solids. Despite the comparatively high concentration in chloride the quality here is considered to be the best in the area. The water beneath the Nipomo upland has similar chemical composition.

The mingling of waters from the various sources occurs principally beneath the Orcutt upland', where waters moving southward from the Santa Maria plain and westward from the Sisquoc Valley mix with the waters derived from rainfall along the south side of the area. The concentrations of all three constituents decrease towards the center of the Orcutt upland where they reach a minimum, but westward appear to increase again. Furthermore, the concentrations are believed to increase with depth. Water moving southward from the Nipomo upland mingles with that originally derived from the Santa... Maria River. As a result, in this locality there is a southward increase in chloride content, in hardness, and in total solids.

POSSIBILITY OF SEA. WATER ENOROACHMENT

The chloride content of the water is of specific importance in wells Copy of document found at www.nearleth eip coast methore, although there has always been a favorable

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seaward gradient, some alarm has been expressed with respect to sea-water encroachment. Analyses of samples collected from the wells at the extreme west end of the valley, both in the vicinity of Oso Flaco Lake and in the area west of Guadalupe, show that in 1941 and 1942 the chloride content was between 30 and 60 parts per million. Furthermore, analyses made in 1927 show about the same range. These chloride concentrations are far within the limits of safe use. and do not indicate any sea-water encroachment. Furthermore, as discussed below, hydraulic conditions at the coast are such that seawater encroachment presents no immediate threat to water pumped by wells.

In order to determine where the contact between the fresh water and salt water in the permeable deposits along the coast might be at the present time it is necessary to apply the so-called Ghyben-Herzberg theory as used by Brown (1925) in ground-water investigations along the Connecticut coast. Fundamentally the principal involved deals with the density differential between fresh and salt water. In proportion to the slightly greater density of sea water the contact between the two will be depressed about 40 feet below sea level for each foot of fresh-water head above sea level, assuming the specific gravity of the sea water to be 1.025 .

It has been shown that in 1944 the fresh-water head at the coast. as projected westward from the gradient determined by water levels in wells, was about 30 feet above sea level (pl. 6). Therefore, it can be calculated that the contact between fresh water and salt water is theoretically about 1,200 feet below sea level at the shore line. Because the deposits at the coast attain a maximum thickness of roughly 1,500 feet along the axis of the Santa Maria syncline, the salt water theoretically extends inland about 2 miles in the form of a narrow tongue, and its contact with the overlying fresh water plunges downward inland until it intersects the surface of the consolidated rocks at a depth of about 1,600 feet below sea level.

In 1936, when the head was the minimum of record, or about 20. feet, the salt-water contact may have been about 800 feet below sea level at the coast, and theoretically intersected the surface of the consolidated rocks along the axis of the syncline approximately 4 miles inland and at a depth of about 1,800 feet. At any time the theoretical computations would represent about the true conditions if the water-bearing materials were homogeneous throughout, and if movement were instantaneous. Each of these factors, however, is important in controlling the actual position of the contact between fresh water and salt water.

Owing to the lenticular nature of the deposits forming the Paso Robles formation, water is enabled to move more freely along lines parallel to the lenses than vertically across the lenses. Thus, throughout the lower and by far the greater part of the cross-sectional area (section $D-D'$, pl. 2), the natural seaward movement of ground water has probably established the contact at a point farther westward than it would be in homogenous material. No way is now available to determine the amount by which the contact is adjusted within these deposits, but it is obvious that this natural adjustment is favorable to the fresh-water supply.

Also, in deposits such as those at the coastal edge of the Santa Maria Valley the rate of movement of ground water is commonly not more than a few hundred feet a year. Thus, it seems obvious that, following a lowering of water level similar to the one culminating in 1936, a period of many years would elapse before inland and upward movement of the saline contact could bring salt water to its theoretical position under the head relationship. For this reason the inland advance that was developing into 1936 as a result of lowered water levels must have been reversed by the rising water levels of the years following 1936 long before sea water could have far invaded the area. Since 1936, seaward retreat of the salt-water contact doubtless has occurred but probably has not achieved balance with the higher water levels.

The following can be concluded with respect to sea-water encroachment: The salt-water contact lies at considerable depth beneath and west of the bottom of the deepest water wells. Specifically, within the range of the deposits tapped by wells the contact probably lies off shore, which would be several miles from the westermost irrigation well. The head at the coast can be reduced to or even somewhat below that of 1936 without creating a hazard to the fresh-water supply. A considerable depletion of storage would be necessary in order to bring the salt-water contact into the westermost wells. Finally, at the present time the head at the coast and the quantity of outflow are more than sufficient to maintain the salt-water contact at a safe distance from wells.

SELECTED WELL LOGS

Table 16 contains 100 logs of water wells-about one-fifth of the total available in the Santa Maria Valley area. They have been selected to give as complete an areal coverage as possible, to show the range in depth of wells, and to indicate the lithologic character of the stratigraphic units penetrated by the wells. Copy of document fot # 影響 (www.NoNewWipTax.com

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TABLE 16 - Drillers records of wells in the Santa Maria Valley area

[Stratigraphie correlations by G. F. Worts, Jr. Altitudes approximate and with respect to sea-level datum 0.1929

9/32-7A1. Ellen Elliot. On alluvial terrace. Altitude 470 feet [Casing perforated 107 to 115, 154 to 189, 235 to 238, 282 to 290, 410 to 416, and 428 to 436 feet]

9/32-7N1. Valerio Tognazzini. On the Sisquoc plain. Altiude 422 feet [Casing perforated 82 to 97, 105 to 145, and 162 to 185 feet]

9/32-17K1. E. C. Lyman. On Sisquoc plain. Altitude 454 feet

[Oasing perforated 51 to 58, 269 to 279, 317 to 318, 365 to 370, 392 to 402, 410 to 415, and 423 to 426 feet]

SELECTED WELL LOGS

TABLE 16.-Drillers records of wells in the Santa Maria Valley area-Continued

9/32-18A1. Maria Dutra. On Sisquoc plain. Altitude 433 feet [Casing perforated 50 to 60, 78 to 81, 90 to 95, 160 to 162, 206 to 208, 330 to 360, and 388 to 406 feet]

9/32-24E1. Sisquoc Investment Co. On Sisquoc piain. Altitude 545 feet

[Casing perforated 15 to 46 feet]

9/33-1L1. M.V. Diaz. On Sisquoc plain. Altitude 391 feet

[Casing perforated 90 to 115, 125 to 132, 175 to 180, 200 to 230, and 244 to 288]

9/33-2A1. Santa Maria Realty Co. On Sisquoc plain. Altitude 379 feet

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TABLE 16.—Drillers records of wells in the Santa Maria Valley area—Continued 9/33-5B1. Bradley Land Co. On Orcutt upland. Altitude 453 feet.

	Thick- ness (feet)	Depth (feet)		Thick- $_{\text{ness}}$ (feat)	Depth (feet)
Dune sand: Soil. Hard pan Sand Sand, fine, water-bearing. Orcutt formation: Upper member: . Sand and streaks of hard pan ₋₋₋₋ - Lower member: and Boulders, gravel, streaks of clay	2 $\overline{2}$ 81 و، 76 20	35 44 120 140	Paso Robles formation: $Clay$ ₋₋₋₋₋₋₋₋₋₋₋₋₋₋₋₋ ----- Clay and gravel Clay and streaks of gravel. Sand Sand, water-bearing Clay G ravel	42 $38 -$ 29 I٥ 23 (7)	182 220 227 256 265 275 298 301 $301 +$

9/33-8K1. K. B. Norswing. On Orcutt upland. Altitude 697 feet

9/33-12B1. Frank Gonsalves. On Sisquoc plain. Altitude 400 feet

[Gasing perforated 58 to 88, 165 to 175, and 180 to 195 feet]

9/33-15D1. South Basin Oil Co. In Bradley Canyon. Altitude 584 feet

[Casing perforated 348 to 350 feet]

SELECTED WELL LOGS

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TABLE 16. - Drillers records of wells in the Santa Maria Valley area - Continued 9/34-3A2. War Department, Santa Maria Army Air Base. On Orcutt upland, Altitude 271 feet [Casing perforated 247 to 251, 258 to 271, and 284 to 331 feet]

	Thick- ness $_{\rm (feet)}$	$\mathop{\rm Depth}\nolimits$ (feet)		Thick- $_{\text{ness}}$ (feet)	$\mathop{\rm Depth}\nolimits$ (feet)
$\ddot{}$ Orcutt formation: Upper member: Top soil Hard pan, sandy $Sand$ ₋₋₋₋₋₋₋₋ Gravel, not water-bearing. $No record$ ₋₋₋₋₋₋₋₋₋₋₋₋₋₋₋₋ Lower member: Gravel, large and boulders. (Paso Robles (7) formation. Sand, coarse, water-bear- Sand, fine, solid, white Paso Robles formation: Clay, gravel, and sand "Hard pan" and elay, . sandy, white	3 38 R 17 40 13 46 12 35 8 . 6	з 41 49 66 166 119 165 177 212 220 $226 -$	Paso Robles formation--Con. Clay, (gumbo), hard, $gray$ ₋₋₋₋₋₋₋₋₋₋₋ Gravel and day Clay, sandy, and gravel Gravel, loose_____________ Gravel and clay----------- Gravel and sand Clay yellow Clay and gravel Gravel, loose Sand and gravel Clay and gravel Sand and grave Clay, yellow-	10° \cdot 1 10 : 4 11 13 ٠g 12 9 19	236 237 247 251 258 260 271 284 293 302 314 323 331 350

9/34-3N4. City of Santa Maria. On Orcutt upland. Altitude 255 feet

[Casing perforated 481 to 483, 580 to 595, 681 to 684, 701 to 719, 780 to 786, 801 to 804, and 874 to 880 feet]

9/34-4F1. War Department, Santa Maria Army Air Base. On Orcutt upland. Altitude 225 feet

[Casing perforated 259 to 267, 310 to 328, and 337 to 375 feet]

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TABLE 16.-Drillers records of wells in the Santa Maria Valley area-Continued 9/34-10J3. Ida A. Twitchell., On Orcutt upland. Altitude 361 feet

[Casing perforated 378 to 391 feet]

9/34-15B1. County of Santa Barbara, Orcutt Union School District. On Orcutt upland. Altitude 355 feet

10/33-18C1. La Brea Securities Co. On Santa Maria plain. Altitude 267 feet

[Casing perforated 115 to 140, 300 to 338, 341 to 363, and 395 to 415 feet]

. 10/33-18G1. La Brea Securities Co. On Santa Maria plain. Altitude 273 feet

[Casing perforated 132 to 142, 288 to 320, 336 to 340, and 408 to 422 feet]

SELECTED WELL LOGS

TABLE 16.-Drillers records of wells in the Santa Maria Valley area-Continued 10/33-18H1. La Brea Securities Co. On Santa Maria plain. Altitude 276 feet [Casing perforated 135 to 145, 230 to 245, 242 to 248, 255 to 260, 285 to 305, and 395 to 414 feet]

10/33-18H2. La Brea Securities Co. On Santa Maria plain. Altitude 272 feet

[Casing perforated 126 to 150, and 310 to 317 feet]

 $18\,$

150

Gravel and sand
Boulders and sand
Paso Robles formation:

145

8

 $\frac{48}{21}$

Clay and gravel...

Clay, tough, and gravel.

TABLE 16.—Drillers records of wells in the Santa Maria Valley area—Continued 10/33-21F2. M. J. Santos, On Santa Maria plain. Altitude 312 feet

[Casing perforated 90 to 140, 170 to 203, 243 to 254, 274 to 310, and 320 to 337 feet]

10/33-21R1. L. H. Adam. On Santa Maria plain. Altitude 323 feet

[Casing perforated 95 to 104, and 116 to 150 feet]

10/33-27R1. Newhall Land and Farming Co. On Santa Maria plain. Altitude 353 feet

[Oasing perforated 130 to 224 feet]

SELECTED WELL LOGS

TABLE 16.-Drillers records of wells in the Santa Maria Valley area-Continued 10/33-28A1. Joe Soares. On Satita Maria plain. Altitude 325 feet

[Casing perforated 100 to 215, and 242 to 335 feet]

10/33-30L1. R. R. Bush Oil Co. On Orcutt upland. Altitude 310 feet

[Casing perforated 190 to 210, 218 to 244, 268 to 286, 310 to 315, 327 to 342, 335 to 418, and 450 to 485 feet]

10/33-33H1. E. L. Sargent. On Orcutt upland. Altitude 402 feet

[Casing perforated 204 to 232, 245 to 250, and 270 to 280 feet]

147

140

178
181
196
204
246
242
277
278
290

TABLE 16.-Drillers records of wells in the Santa Maria Valley area-Continued 10/33-34H1. Dan Donovan. On Santa Maria plain. Altitude 352 feet

[Casing perforated 70 to 80, 86 to 145, 160 to 191, 220 to 234, 237 to 238, 257 to 265, 275 to 282, and 293 to 300 feet]

SELECTED WELL LOGS

TABLE 16.-Drillers records of wells in the Santa Maria Valley area-Continued 10/34-2R1. Gracio Apalatequi. On Santa Maria plain. Altitude 230 feet [Ossing perforated 106 to 130, 180 to 190, and 221 to 226 feet]

Thick-Thick. Depth $\frac{\text{mes}}{\text{fest}}$ $Depth$ ness
(feet) $(t$ eet) (teet) Alluvium: Paso Robles formation-Con. Upper member: Gravel................... \mathbf{I} 222 Soil...................... Olay and gravel........... Sand and gravel $\overline{2}$ 224 96 100 $\overline{2}$ 226 Lower member: ıō $\frac{220}{236}$ 29 129 Sand and some gravel.... $\frac{1}{4}$ 240 Careaga (?) sand: Clay............. 46 175 Sand, blue......... 15 255 Clay and gravel......... 182 Olay, blue 20 275 Gravel.................. - 5 187 Sand $\overline{3}$ 278 Clay and gravel.......... \mathcal{R} 195 Clay and sand............ 14 292 26 221 Sand $\overline{2}$ 294 10/34-5R1. La Brea Securities Co. On Santa Maria plain. Altitude 175 feet [Oasing perforated 125 to 158, and 192 to 238 feet] Alluvium; Alluvium-Continued Upper member: Lower member-Continued
Sand, hard-Soil 10 10 118 Sand, fine, and clay...... 52 62 Gravel
Paso Robles formation: $4\overline{1}$ 159 Sand, coarse, and gravel. $\frac{57}{24}$ $^{86}_{90}$ $\frac{4}{5}$ Olay and sand 19. 178 Olay and gravel.......... 55 Sand and a little gravel. $_{\text{Lower member}}$: 12 190 Gravel and boulders. $\overline{48}$ Gravel...... 238 100 Sand................... 12 Sand.................... 250 16 116 10/34-7F1. Antone Souza. On Santa Maria plain. Altitude 161 feet [Casing perforated 104 to 108, 121 to 168, and 195 to 198 feet] Alluvium: Allugium-Continued Upper member: Lower member-Continued Loam, sandy.............. -6 Gravel, coarse............. -6 33 153 52° 58 154 $\frac{1}{14}$ $\overline{3}$ 61 Gravel.................... 168 28 $\frac{89}{97}$ Paso Robles formation: $\overline{8}$ Gravel and clay. $\overline{2}$ 170 Olay
Lower member: $\overline{4}$ 101 $Clay$ $\boldsymbol{28}$ 198 Boulders and clay Gravel and clay....... $\frac{2}{19}$ 200 9 110 Clay, yellow $.219$ 10 120 Clay, yellow 220 10/34-8H1. Mrs. Virgil Alexander. On Santa Maria plain. Altitude 176 feet [Casing perforated 103 to 152 and 183 to 200 feet] Alluvium: Alluvium-Continued Upper member: Lower member-Continued
"Hardpan"-Loam................... \cdot .10 10 Sand---------------------- \cdot ₃ 103 30 $40[°]$ Gravel, coarse, water- $\begin{array}{c} 10 \\ 20 \\ 17 \end{array}$ $50\,$ bearing
Paso Robles formation: 49 152 Sand 70 "Quicksand".............. 87 Clay, yellow, soft $\bf 28$ Clay, sandy---------------180 $\overline{\mathbf{3}}$ 90 Sand, hard................. Clay and gravel.......... 183 $\overline{4}$ 94 Gravel................... Lower member: 17 200 Clay................... Gravel, coarse.... -2 202 100

particulary.

TABLE 16.-Drillers records of wells in the Santa Maria Valley area-Continued 10/34-9D1. J. Rembush. On Santa Maria plain. Altitude 183 feet.

[Casing perforated 110 to 151 and 180 to 230 feet]

10/34-10E1. L. O. Donati. On Santa Maria plain. Altitude 198 feet [Casing perforated 136 to 221 feet]

10/34-12E1. C. C. Mitchell. On Santa Maria plain. Altitude 230 feet

[Casing perforated 130 to 147 feet]

10/34-12J1. F. N. Silva. On Santa Maria plain. Altitude 255 feet

[Casing perforated 110 to 114, 125 to 155, 192 to 206, 224 to 228; and 233 to 237 feet]

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SELECTED WELL LOGS

TABLE 16. - Drillers records of wells in the Santa Maria Valley area - Continued 10/34-13G1. La Brea Securities Co. On Santa Maria plain. Altitude 253 feet

[Casing perforated 136 to 160, 165 to 170, 344 to 350, 363 to 373, and 390 to 395 feet]

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10/34-14E3. City of Santa Maria. On Santa Maria plain. Altitude 225 feet

[Casing perforated 87 to 109 and 164 to 181 feet]

10/34-16N1. E. and G. LeRoy. On Santa Maria plain. Altitude 187 feet

[Casing perforated 89 to 119, 135 to 160, and 175 to 206 feet]

Alluvium-Continued
Lower member-Oontinued
Clay and gravel............
Gravel and boulders..... Alluvium: Upper member:
Clay......... -6 $17\,$ 152 Sand........... 71 $^{77}_{89}$ 8 160 Paso Robles formation. Clay and gravel.... 12 Lower member: Clay
Gravel and boulders. $15\,$ 175 Gravel and boulders. 119 35° 210 Sand 16 135

10/34-18D1. Dan Donovan. On Santa Maria plain. Altitlude 147 feet

TABLE 16.-Drillers records of wells in the Santa Maria Valley area-Continued 10/34-18P1. Olga Giacomini. On Santa Maria plain. Altitude 154 feet

Casing perforated 95 to 120, 155 to 180, and 188 to 234 feet]

SELECTED WELL LOGS

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TABLE 16.-Drillers records of wells in the Santa Maria Valley area-Continued 10/34-23L2. (K.A. Newlove, On Santa Maria plain., Altitude 232 feet [Casing perforated 168 to 214 and 226 to 266 feet]

Thick-Thick, ${\tt Depth}$ res_s
(feet) $Depth$ $(feet)$ ness. $(\hat{f}e\hat{e}t)$ $($ feet $)$ Alluvium: $\hbox{\it Alluvium}-\hbox{\it Continued}$ Upper member: Lower member-Continued Soil________ Gravel and sand. 8 Sand....... 158 31 36 Paso Robles formation; Clay and gravel......... $\overline{40}$ $\overline{4}$ Gravel and boulders, Gravel............. 40 $\overline{80}$ comented______________ $\begin{array}{c} 42 \\ 14 \\ 10 \\ 4 \\ 38 \end{array}$ Lower (?) member: 200 Clay and gravel......... 200
214
224
228
266
270 Sand and gravel $20\,$ 100 $Clay$ ₋₋₋₋₋₋₋-----Lower member: --------Olay and gravel........ Clay, hard, and boulders_ $2\mathrm{\dot{0}}$ 120 126 -6 Clay..................... Clay and gravel.......... 24 150

10/34-24KI. Union Oil Company of California. On Santa Maria plain. Altitude 254 feet

[Oasing perforated 650 to 657 and 692 to 710 feet]

10/34-27J1, J. Morrison. On Orcutt upland. Altitude 246 feet

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TABLE 16.-Drillers records of wells in the Santa Maria Valley area-Continued 10/34-28 Cl. Stephen Nicholai. On Orcutt upland. Altitude 215 feet

[Casing perforated 188 to 191 feet]

SELECTED WELL LOGS

TABLE 16 -- Drillers records of wells in the Santa Maria Valley area-Continued 10/35-5J1. Union Sugar Co. On Santa Maria plain. Altitude 79 feet

155

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SELECTED WELL LOGS

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TABLE 16.-Drillers records of wells in the Santa Maria Valley area-Continued'

: 11/35-20E1. Union Sugar Co. On Santa Maria plain. Altitude 49 feet

[Casing perforated 150 to 444 feet]

11/35-21R2. B. A. Tognazzini. On Santa Maria plain. Altitude 77 feet [Casing perforated 392 to 415 feet]

11/35-25L1. M. C. Gracia. On Santa Maria plain. Altitude 127 feet

[Casing perforated 95 to 121, 230 to 250, 267 to 275, 295 to 300, 311 to 313, and 325 to 335 feet]

SELECTED WELL LOGS

TABLE 16.-Drillers records of wells in the Santa Maria Valley area-Continued

11/35-25P1. Dave McKeen. On Santa Maria plain. Altitude 126 feet

[Casing perforated 110 to 179 feet]

11/35-27H1. Henry Tognazzini. On Santa Maria plain. Altitude 101 feet

11/35-28L1. Union Sugar Co. On Santa Maria plain. Altitude 84 feet

[Casing perforated 273 to 278 and 290 to 390 feet]

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TABLE 16.-Drillers records of wells in the Santa Maria Valley area-Continued

11/35-29R1. Union Sugar Co. On Santa Maria plain. Altitude 85 feet

[Casing perforated 144 to 154, 210 to 223, 310 to 322, and 390 to 428 feet]

11/35-33F1. Union Sugar Co. On Santa Maria plain. Altitude 84 feet

[Casing perforated 118 to 150, 154 to 170, and 174 to 208 feet]

11/35-35A1. Bello Estate. On Santa Maria plain. Altitude 123 feet

[Casing perforated 125 to-189 feet]

REFERENCES CITE

WELL-NUMBERING SYSTEM

The well-numbering system used by the Geological Survey in the Santa Maria Valley area shows the locations of wells according to the rectangular system for the subdivision of public land. For example, in the number 10/34-14E3, which was assigned to a well within the city limits of Santa Maria, the part of the number preceding the bar indicates the township (T. 10 N.), the part between the bar and the hyphen, the range $(R. 34 W.)$; the digits between the hyphen and the letter indicate the section (sec. 14), and the letter indicates the 40-acre subdivision of the section shown in the accompanying diagram.

Within each 40-acre-tract the wells are numbered serially as indicated by the final digit of the number. Thus, well 10/34-14E3 is the third well to be listed in the SW\XNW\% sec. 14. As all of the Santa Maria Valley area is in the northwest quadrant of the San Bernardino meridian and base line, the foregoing abbreviation of the township and range is sufficient. The west half of the area has never been public land; for this the rectangular system of subdivision has been projected. The correlation of Geological Survey well numbers with those of other agencies has been presented in another report (La Rocque, Upson, and Worts, 1950, tables 1 and 4.)

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